Estabrook Residence

American Ranch Lot 109



Enont Elevation

Project Information Seet Index

Ron Estabrook 8870 N. Buchanon Prescott, AZ 86301

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PREPARED BY W. Alan Kenson & Assoc., P.C. P.O. Box 11593 Prescott, AZ 86304

Contact: Alan Kenson PH: 928-443-5812 wakaarchitect@gmail.com

9185 N. American Ranch Road **ADDRESS**: Prescott, AZ 86305

PARCEL NUMBERS 100-18-129

PAD **ZONING:**

Residential Residential Group R

CONST. TYPE

CLIENT:

SITE USE:

OCCUPANCY

CURRENT CODE:

2018 International Residential Code 2018 International Fire Code

2006 International Energy Conservation Code

2018 International Plumbing Code 2018 International Mechanical Code 2018 International Fuel Gas Code 2018 International Electrical Code 2017 National Electrical Code

AREA SUMMARY:

1st Floor Livable: 1st Floor Garage: Porch & Patio: Total under roof:

ARCHITECTURAL

Cover Sheet CS₁ **General Notes C.01** Civil Grading and SWPPP Plan (Grading plan already approved under separate permit)

A0.0A1.0 Floor Plan

A1.1 **Dimension Plan** A1.2 Wall Types Plan A2.0

A3.0**Exterior Elevations** A3.1 **Exterior Elevations**

A3.2 **Exterior Elevations**

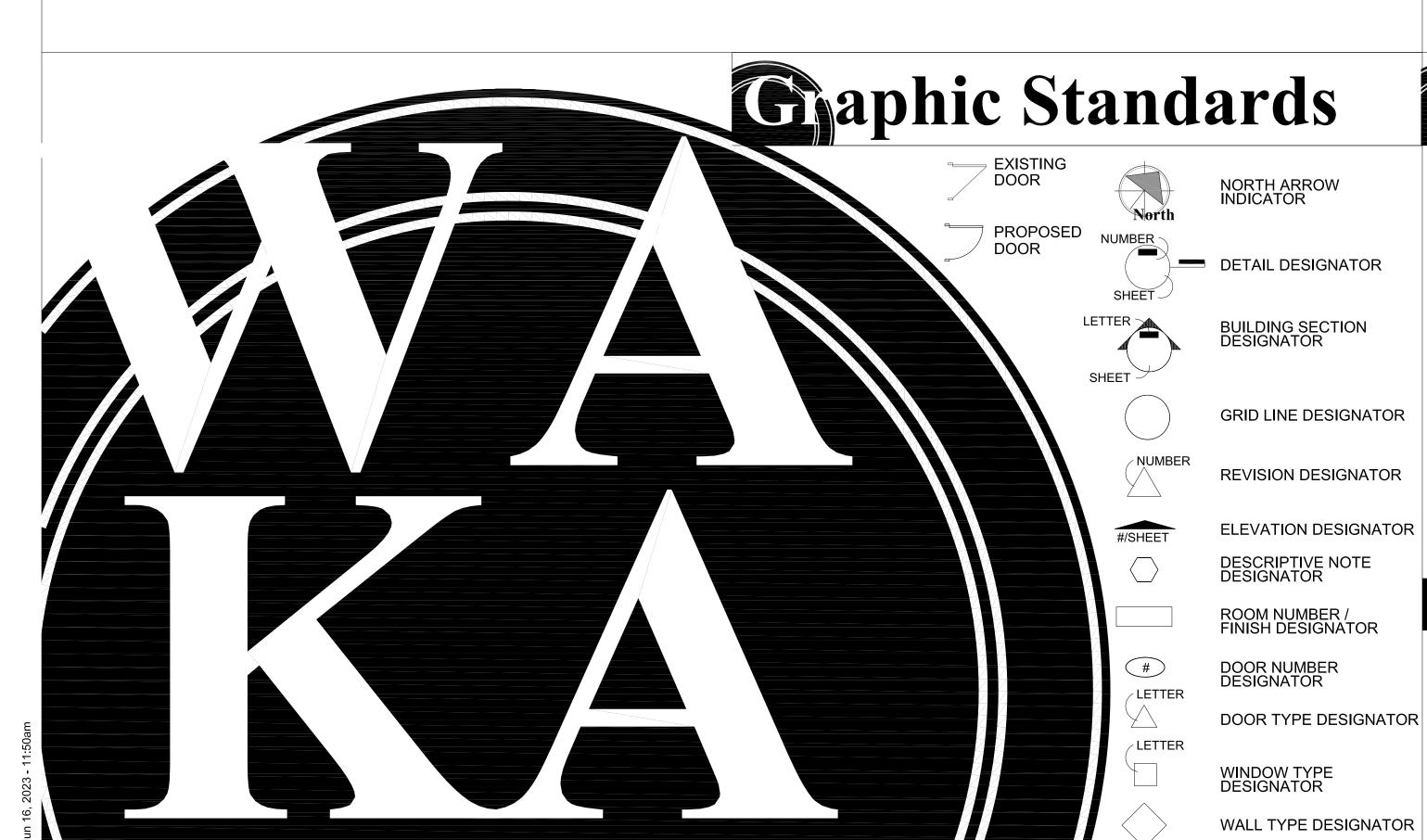
A4.1 Building Sections A4.2 **Building Sections** A5.0**Schedules**

A6.0 **Details** A7.0**Room Finish Plan** A8.0 **Ceiling Framing Plan**

P1.0 Plumbing Plan and Gas Isometric E1.0 **Electrical Power Plan** E1.1 **Lighting Reflected Ceiling Plan** E1.2 **Electrical One-Line Diagram**

STRUCTURAL

General Structural Notes S-1.1 **Typical Details Roof Framing Plan Foundation Details** Framing Details



Micinity Map





Deferred Submittal

FIRE SPRINKLER SYSTEM SHALL BE INSTALLED. REFER TO FIRE

Architect:

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ARCHITECTURE & PLANNING



W. Alan Kenson & Associates P.

REVISIONS

W. Alan Kenson & Associates, P.

L.O. CHECKED BY W.A.K. June 14th, 2023

Estabrook Residence

American Ranch

General Notes

- 1. A COPY OF THE YAVAPAI COUNTY APPROVED CONSTRUCTION DRAWINGS SHALL BE KEPT AT THE JOB SITE.
- 2. EXTERIOR WALLS: CONSTRUCTION, PROJECTIONS, OPENINGS AND PENETRATIONS OF EXTERIOR WALLS OF DWELLINGS AND ACCESSORY BUILDINGS SHALL COMPLY WITH IRC 2018TABLE 302.1.
- CEMENT, FIBER-CEMENT AND GLASS MAT GYPSUM BACKERS SHALL BE USED AS BACKERS FOR WALL TILE IN TUB AND SHOWER AREAS AND WALL PANELS IN • SHOWER AREAS. SUCH WALL SURFACES SHALL EXTEND TO A HEIGHT OF NOT LESS THAN 6 FEET ABOVE THE FLOOR.
- 4. EVERY SLEEPING ROOM AND BASEMENT WITH HABITABLE SPACE SHALL HAVE AT LEAST ONE WINDOW WITH A NET CLEAR OPENING OF 5.7 SQUARE FEET (MIN. 5 SQUARE FEET NET CLEAR OPENING AT GRADE FLOOR), MINIMUM OPENING WIDTH OF 20" MINIMUM OPENING HEIGHT OF 24" AND THE FINISHED SILL HEIGHT SHALL NOT BE MORE THAN 44" ABOVE THE FLOOR, OR PROVIDE EXTERIOR DOOR FOR EMERGENCY EGRESS
- 5. WINDOWS SHALL BE FLASHED IN ACCORDANCE WITH THE MANUFACTURER'S INSTALLATION INSTRUCTIONS.
- 6. GLAZING IN HAZARDOUS LOCATIONS SHALL COMPLY WITH IRC 308.
- 7. ALL INTERIOR AND EXTERIOR GLAZING IN BATHROOMS MUST BE SAFETY GLAZING WHEN THE BOTTOM EDGE IS LESS THAN FIFTY-SIX INCHES ABOVE THE FLOOR LEVEL. (BATHROOM SHALL BE DEFINED AS A ROOM PROVIDED WITH A TUB OR SHOWER.)
- CEILING INSULATION: R-38 CLOSED CELL SPRAY FOAM INSULATION AT TOP CHORD OF TRUSSES.
- 9. WOOD FRAMED WALLS: MINIMUM R-19 UNFACED BATT INSULATION.

- 10. AIR LEAKAGE THE CODE ALLOWS THE USE OF AIRFLOW RETARDERS (HOUSE WRAPS) OR OTHER SOLID MATERIALS AS ACCEPTABLE METHODS TO MEET THIS REQUIREMENT. TO BE EFFECTIVE, THE BUILDING THERMAL SEAL MUST
- IMPERMEABLE TO AIR FLOW.
- CONTINUOUS OVER THE ENTIRE BUILDING ENVELOPE.
- ABLE TO WITHSTAND THE FORCES THAT MAY ACT ON IT DURING AND AFTER CONSTRUCTION.
- DURABLE OVER THE EXPECTED LIFETIME OF THE BUILDING. ALL SEAMS AND EDGES MUST BE SEALED/TAPED PER MANUFACTURER'S SPECIFICATIONS.
- 11. BUILDING THERMAL ENVELOPE THE SEALING METHODS BETWEEN DISSIMILAR MATERIALS SHALL ALLOW FOR DIFFERENTIAL EXPANSION AND CONTRACTION. THE FOLLOWING SHALL BE CAULKED, GASKETED, WEATHER-STRIPPED OR OTHERWISE SEALED WITH AN AIR BARRIER MATERIAL, SUITABLE FILM OR **SOLID MATERIAL:**
- ALL JOINTS, SEAMS AND PENETRATIONS.
- SITE BUILT WINDOWS, DOORS AND SKYLIGHTS.
- OPENINGS BETWEEN WINDOW AND DOOR ASSEMBLIES AND THEIR RESPECTIVE JAMBS AND FRAMING.
- UTILITY PENETRATIONS.
- DROPPED CEILINGS OR CHASES ADJACENT TO THE THERMAL ENVELOPE.
- WALLS AND CEILINGS SEPARATING A GARAGE FROM CONDITIONED SPACES.
- BEHIND TUBS AND SHOWERS ON EXTERIOR WALLS.
- COMMON WALLS BETWEEN DWELLING UNITS.
- OTHER SOURCES OF INFILTRATION.
- 12. FENESTRATION AIR LEAKAGE WINDOW, SKYLIGHT AND SLIDING GLASS DOORS SHALL HAVE AN AIR INFILTRATION RATE OF NO MORE THAN 0.3 CFM PER SQUARE FOOT, AND SWINGING DOORS NO MORE THAN 0.5 CFM. SPECIFICATION SHALL BE LISTED ON THE MANUFACTURER LABEL. ALL WINDOWS AND EXTERIOR DOORS COMPRISING THE BUILDINGS THERMAL ENVELOPE, SHALL HAVE A FENESTRATION U-FACTOR OF NOT MORE THAN .40.

- 13. RECESSED LIGHTING RECESSED LUMINAIRES INSTALLED IN THE BUILDING THERMAL ENVELOPE SHALL BE SEALED TO LIMIT AIR LEAKAGE BETWEEN
- TO PREVENT AIR LEAKAGE TO THE CEILING CAVITY OR UNCONDITIONED SPACE

- 14. ALL CIRCULATING SERVICE HOT WATER PIPING SHALL BE INSULATED TO AT LEAST R-2. ALL NEW RESIDENCES EXCEEDING 1,800 SQUARE FEET WITH TWO OR MORE BATHROOMS SHALL HAVE A CIRCULATING HOT WATER SYSTEM. CIRCULATING HOT WATER SYSTEMS SHALL INCLUDE AN AUTOMATIC OR READILY ACCESSIBLE MANUAL SWITCH THAT CAN TURN OFF THE HOT WATER CIRCULATING PUMP WHEN THE SYSTEM IS NOT IN USE. THERMAL SIPHONING
- 15. A MINIMUM 0.019 INCH, CORROSION RESISTANT WEEP SCREED, WITH MINIMUM VERTICAL ATTACHMENT FLANGE OF 3-1/2 INCHES SHALL BE PROVIDED AT OR BELOW THE FOUNDATION PLATE LINE ON THE EXTERIOR STUD WALL IN ACCORDANCE WITH ASTM C 926. THE WEEP SCREED SHALL BE PLACED A MINIMUM OF 4 INCHES ABOVE THE EARTH OR 2 INCHES ABOVE PAVED AREAS AND SHALL BE OF A TYPE THAT WILL ALLOW TRAPPED WATER TO DRAIN TO THE EXTERIOR OF THE BUILDING. THE WEATHER RESISTANT BARRIER SHALL LAP THE ATTACHMENT FLANGE. THE EXTERIOR LATH SHALL COVER AND TERMINATE ON THE ATTACHMENT FLANGE OF THE WEEP SCREED.
- AREA BY NOT LESS THAT 1/2" GPDW APPLIED TO THE GARAGE SIDE.
- 17. A WATER HEATER RELIEF VALVE SHALL EXTEND OUTSIDE THE BUILDING WITH THE END OF PIPE NOT MORE THAN (2) TWO FEET OR LESS THAN (6) SIX INCHES ABOVE THE GROUND AND POINTING DOWNWARD.
- 18. MECHANICAL SYSTEM PIPING CAPABLE OF CARRYING FLUIDS ABOVE 105 F OR BELOW 55 F SHALL BE INSULATED TO A MINIMUM OF R-2.

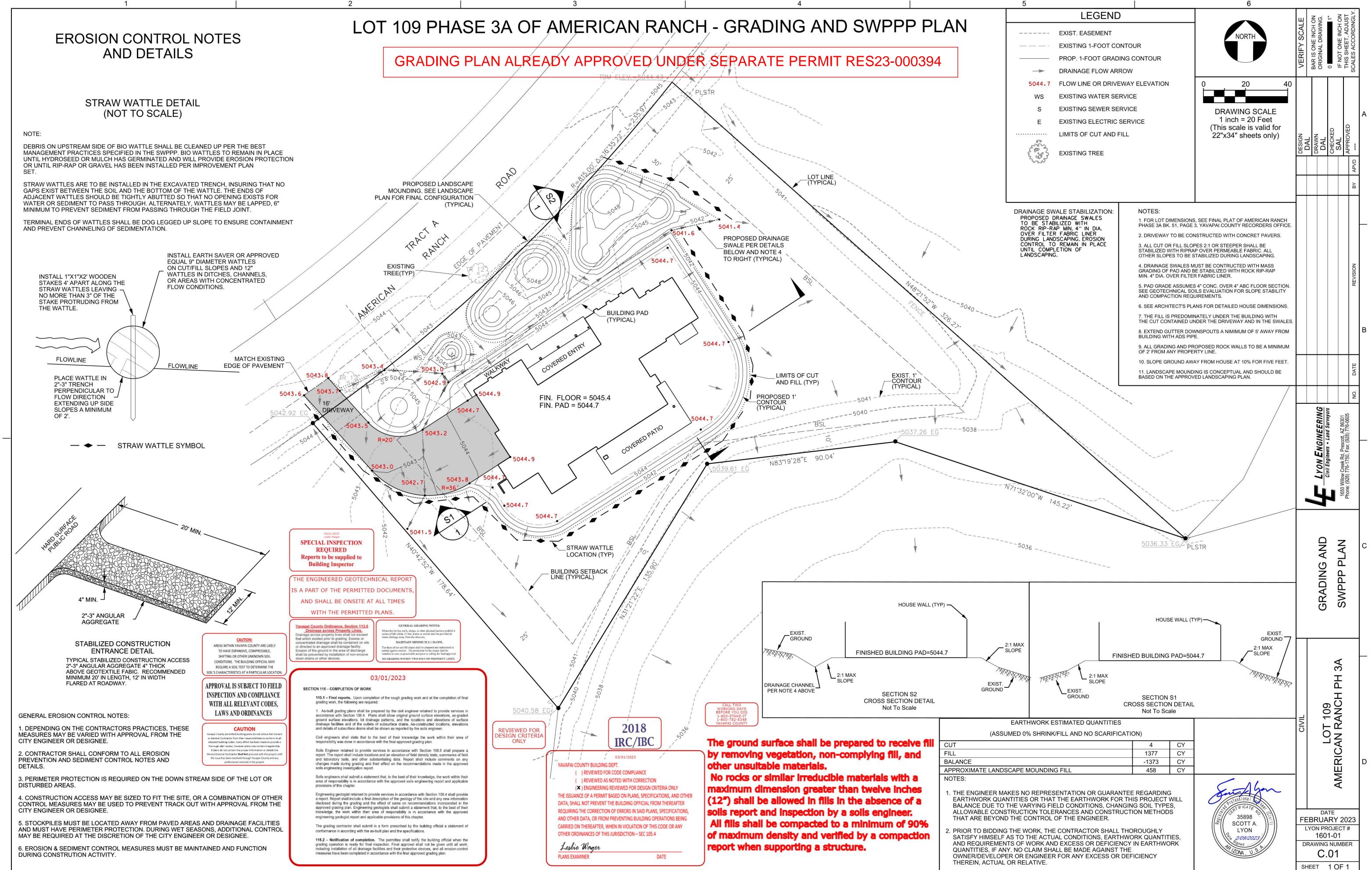
CONDITIONED AND UNCONDITIONED SPACES BY BEING: IC-RATED AND LABELED WITH ENCLOSURES THAT ARE SEALED OR GASKETED

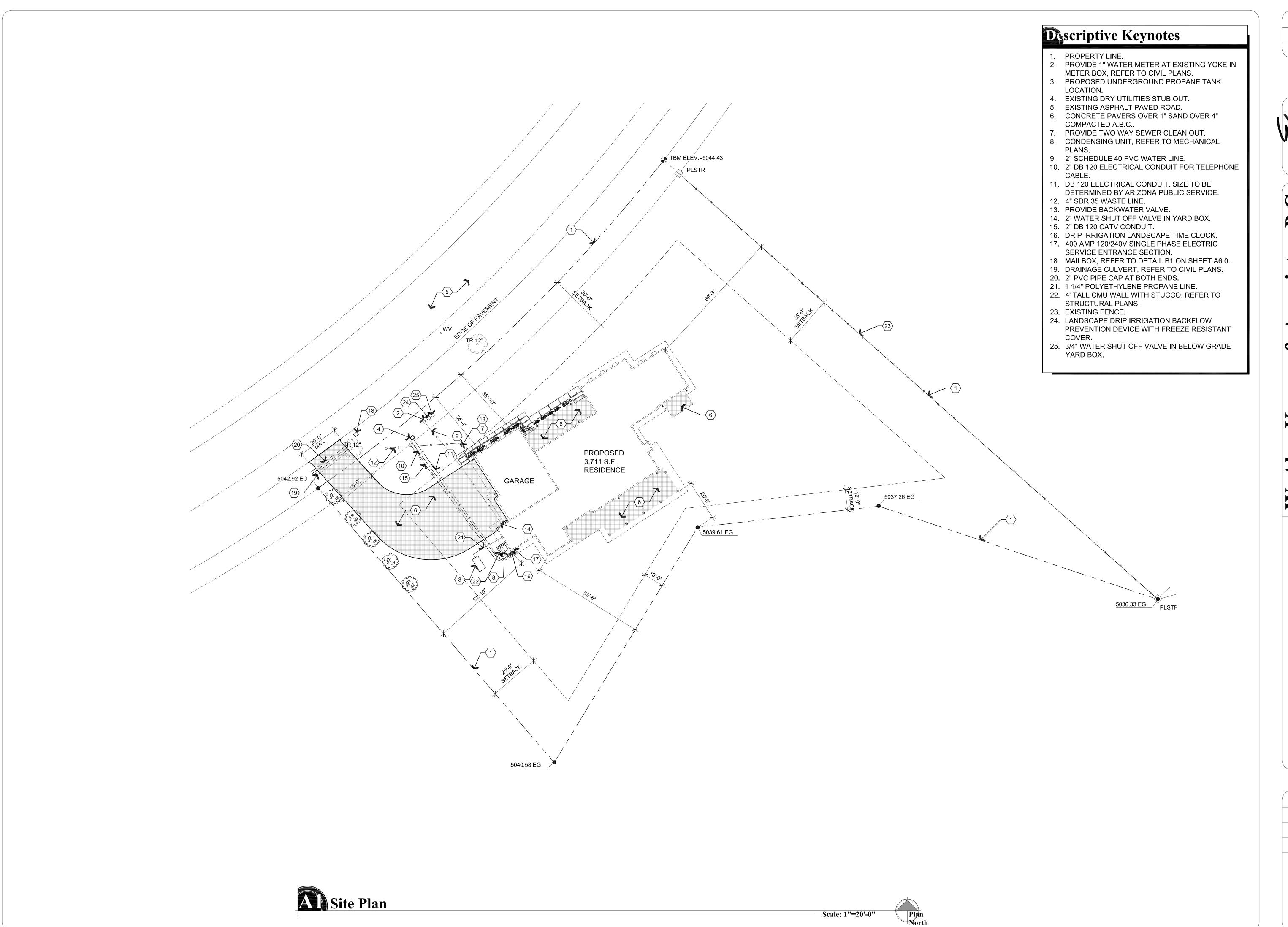
- IC-RATED AND LABELED AS MEETING ASTM E283
- LOCATED INSIDE AIRTIGHT SEALED BOX WITH CLEARANCES OF AT LEAST 0.5 INCH FROM COMBUSTIBLE MATERIAL AND 3 INCHES FROM INSULATION.
- SYSTEMS SHALL HAVE A VALVE TO REDUCE FLOW. ALTERNATE SYSTEM SHALL BE CONSIDERED.
- 16. THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTIC

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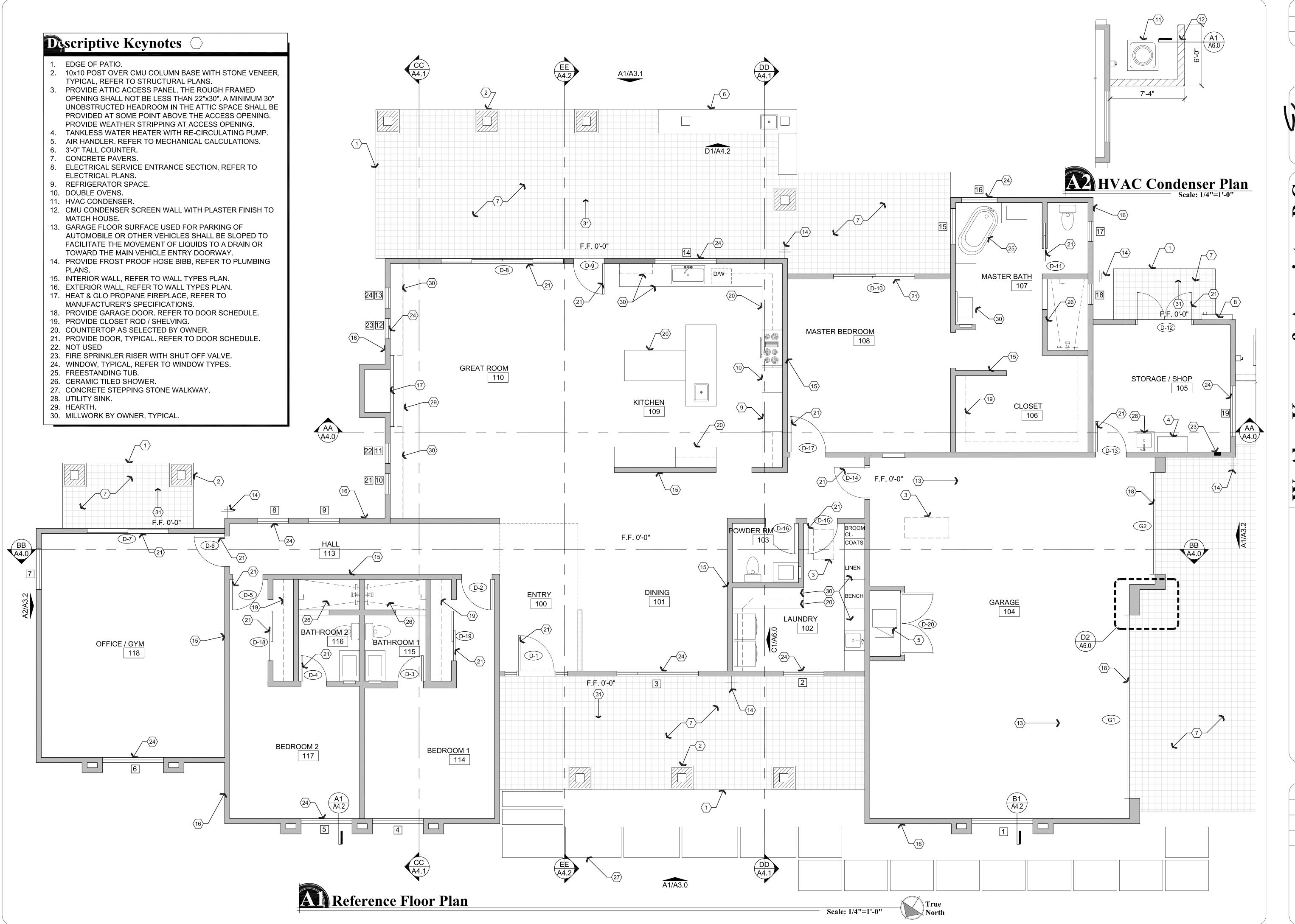
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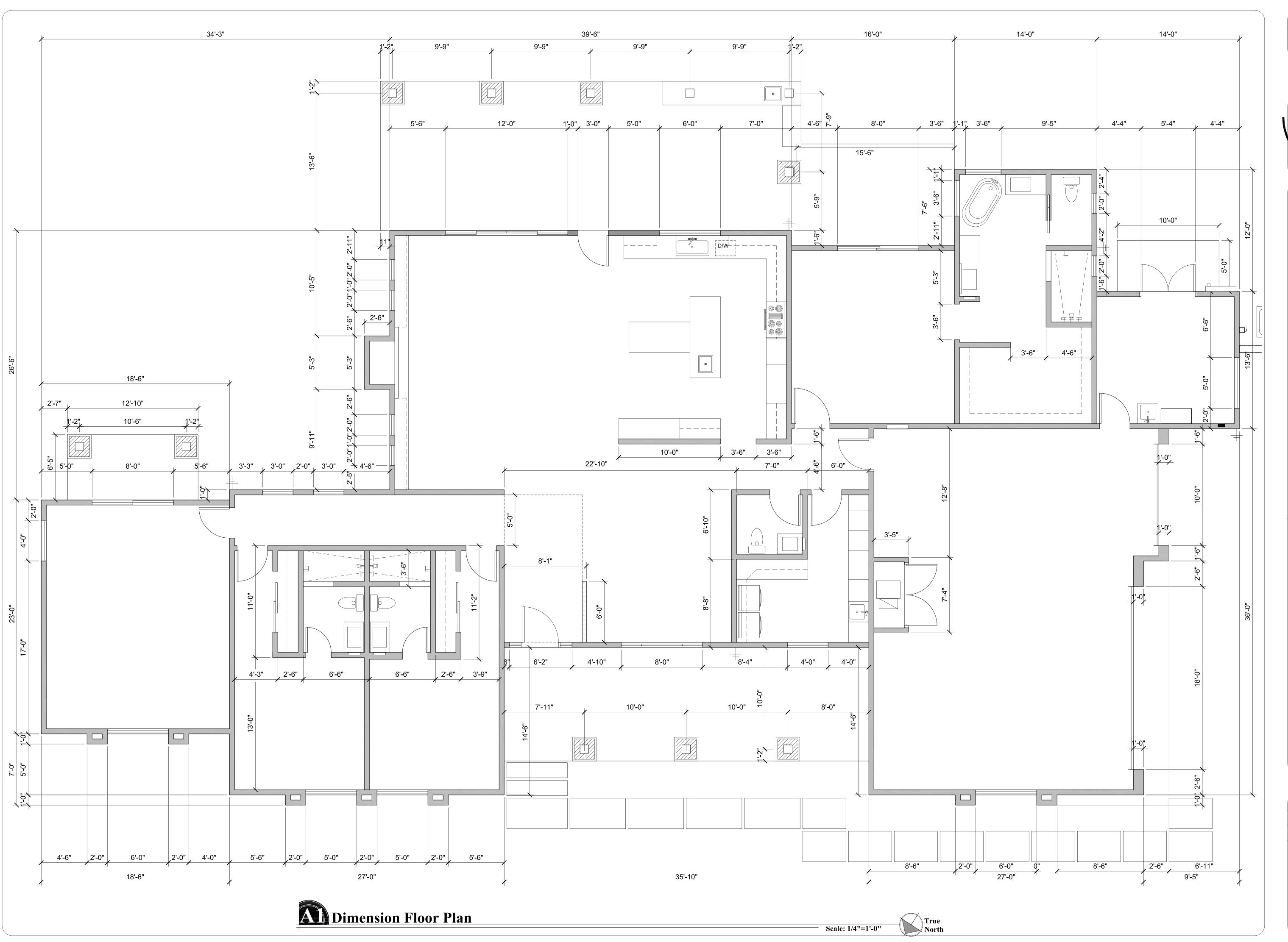
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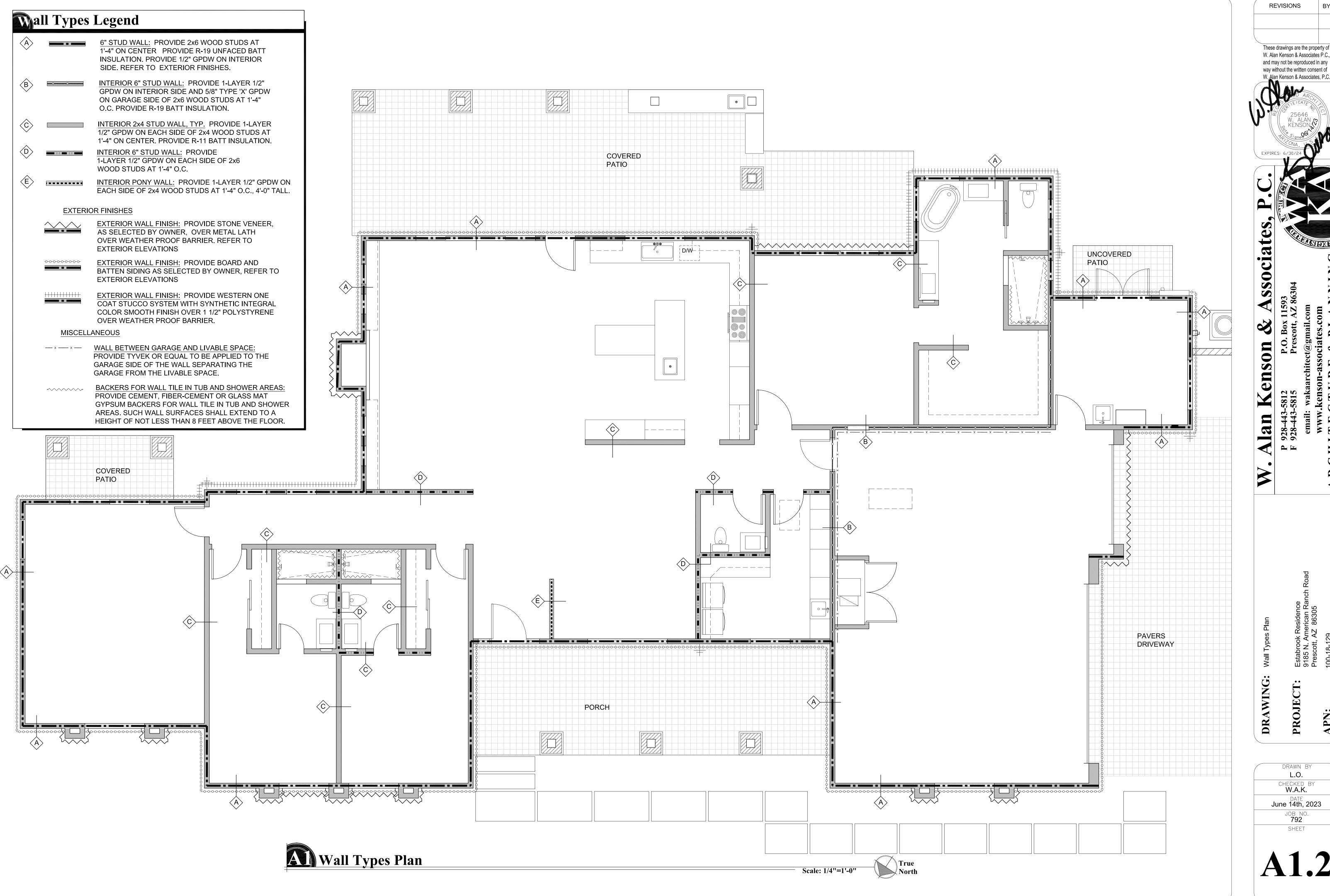
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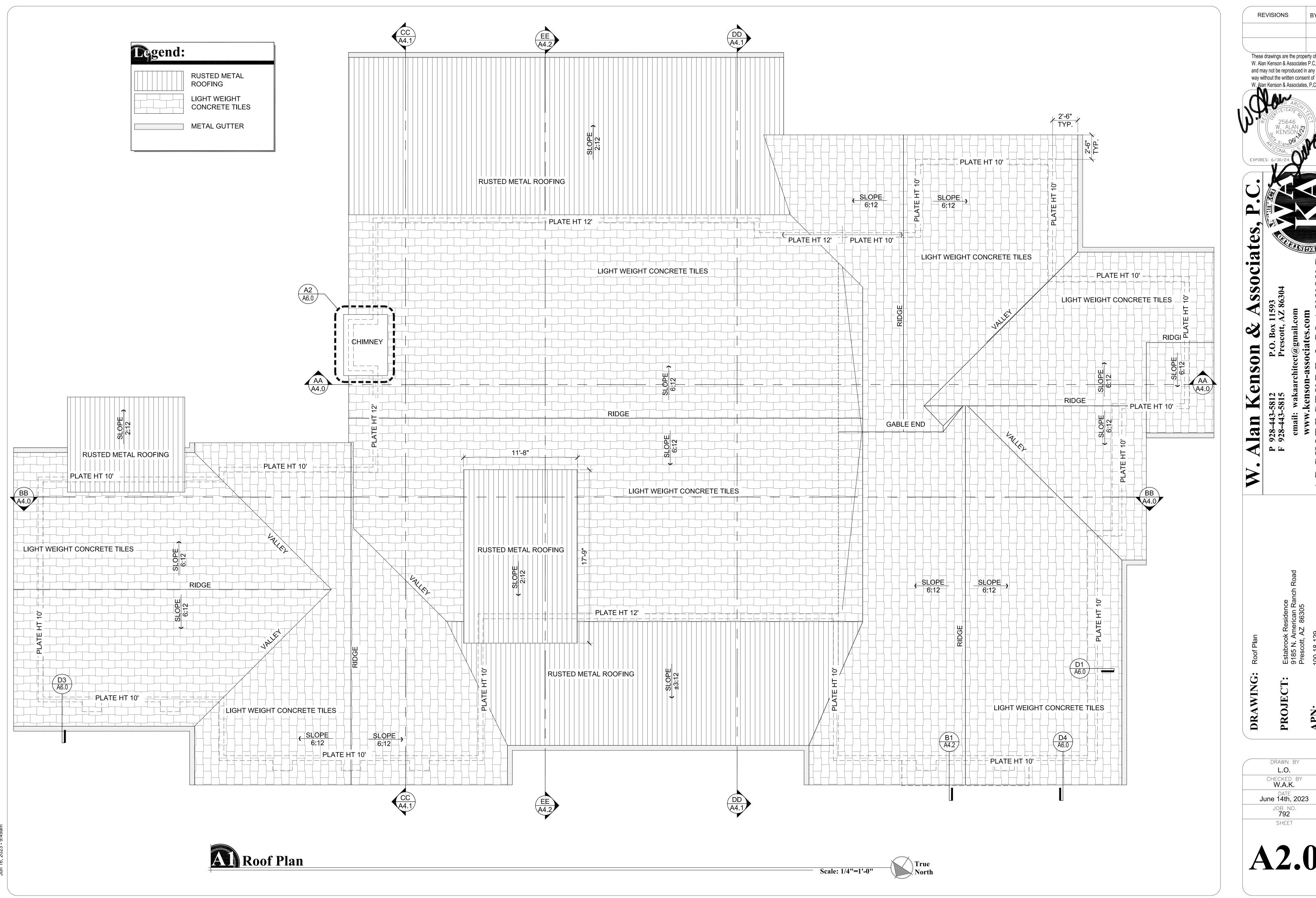
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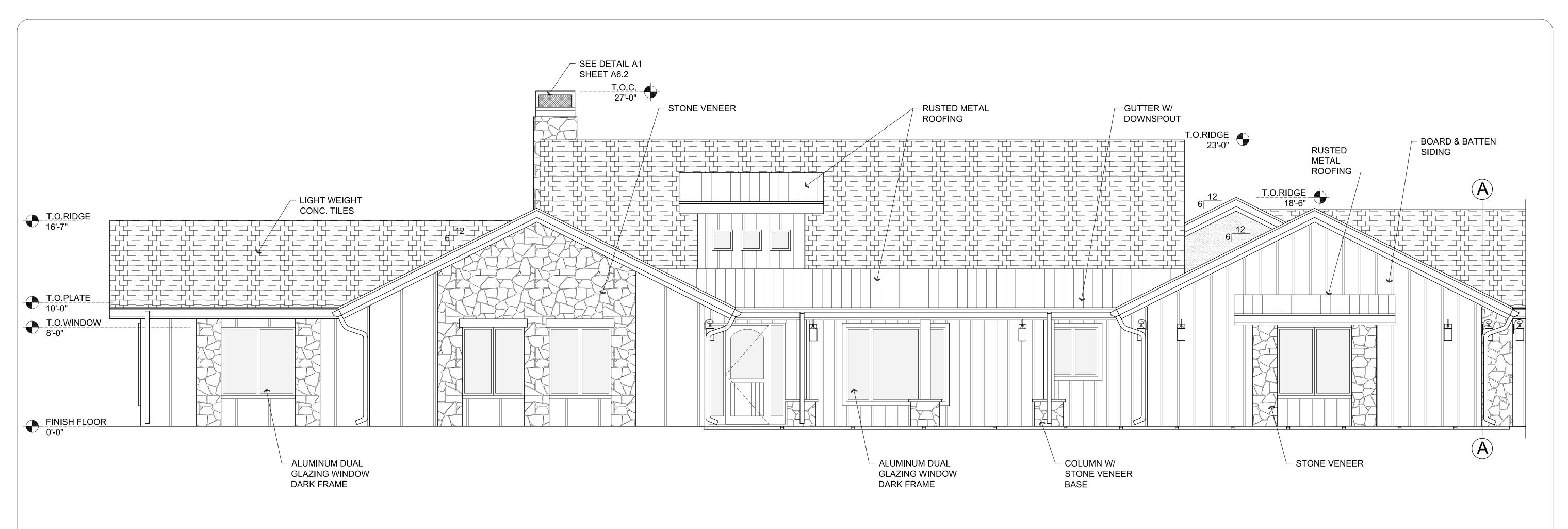
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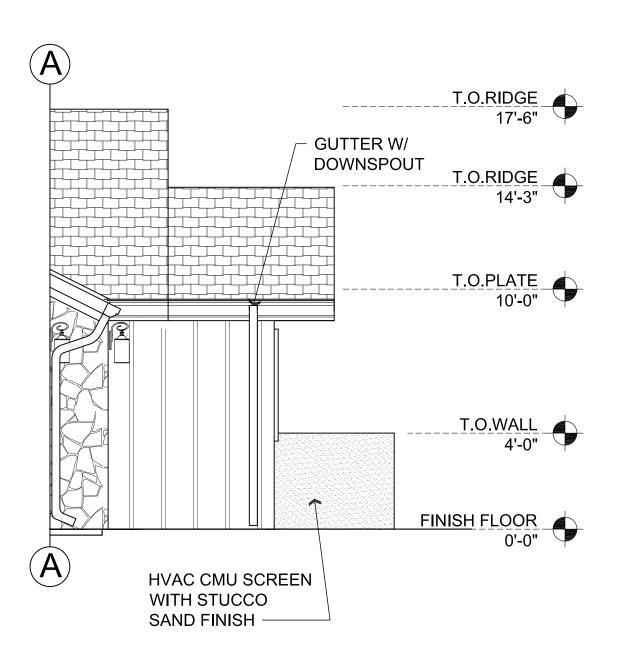
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Front Elevation



Scale: 1/4"=1'-0"

Front Elevation (Right End)
Scale: 1/4"=1'-0"

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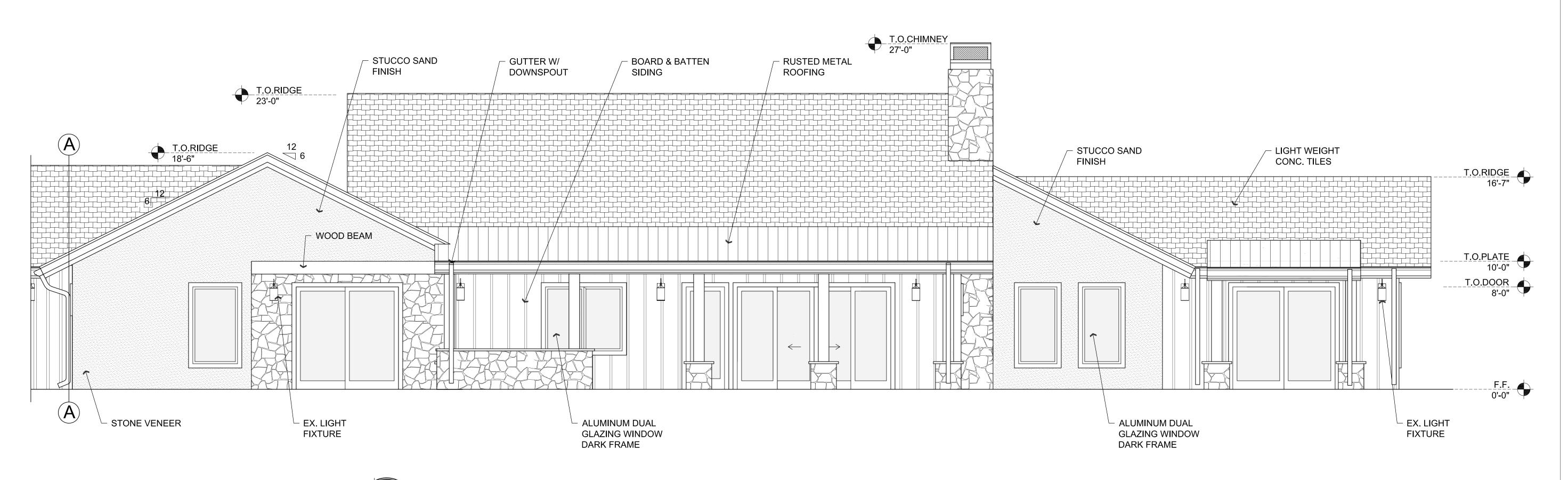
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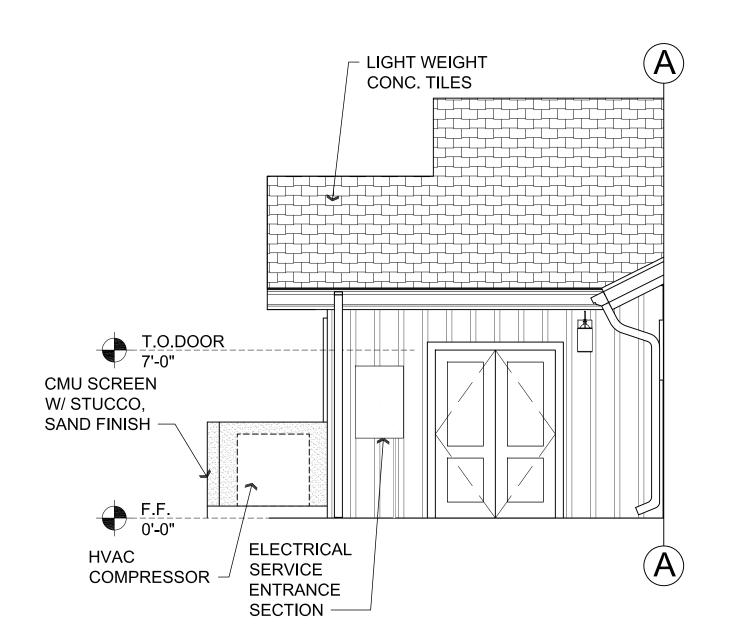
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Rear Elevation

Scale: 1/4"=1'-0"





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AWING: Exterior Elevations

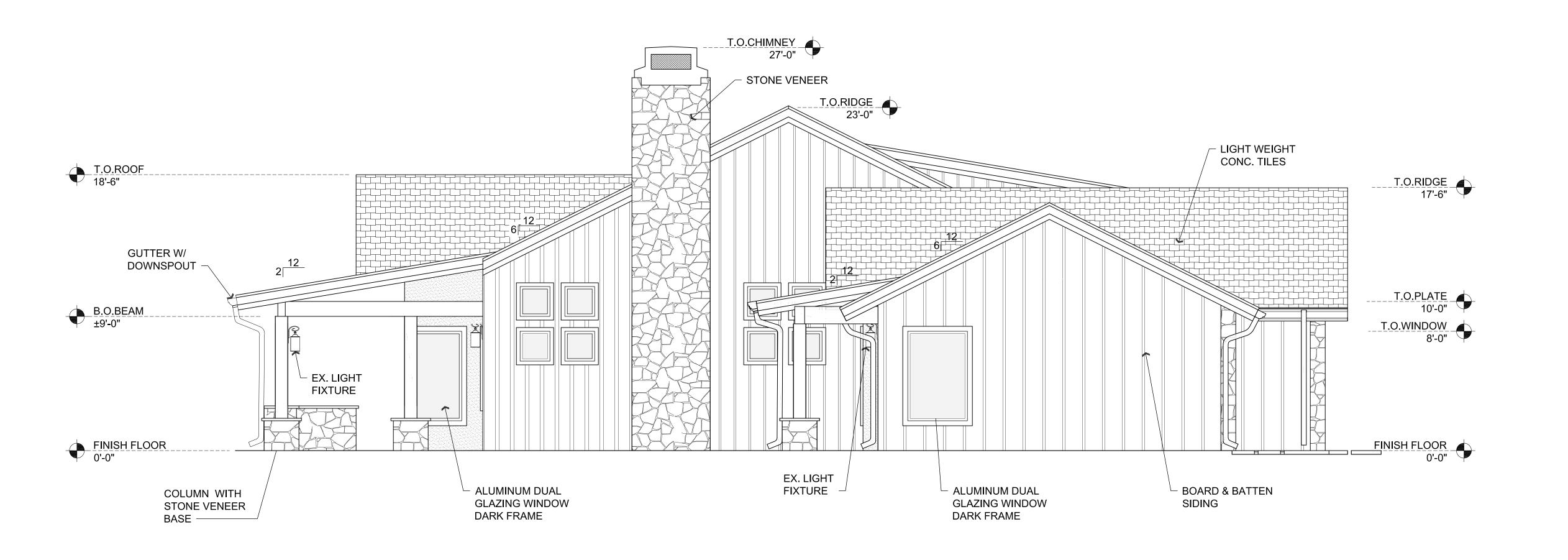
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A3.1



T.O.CHIMNEY 27'-0" BOARD &
BATTEN
SIDING - STUCCO SAND FINISH 2_| CONC. TILES T.O.RIDGE 18'-6" T.O.R. 17'-6" GUTTER W/
DOWNSPOUT B.O.BEAM 9'-0" T.O.GARAGE DOOR 8'-0" EX. LIGHT FIXTURE EX. LIGHT FIXTURE F.F. 0'-0" ALUMINUM DUAL GLAZING WINDOW DARK FRAME GARAGE DOORS - STUCCO SAND FINISH

Right Elevation

Left Elevation

W. Alan Kenson & Associates

Scale: 1/4"=1'-0"

Scale: 1/4"=1'-0"

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9185 N. American Ranch Road

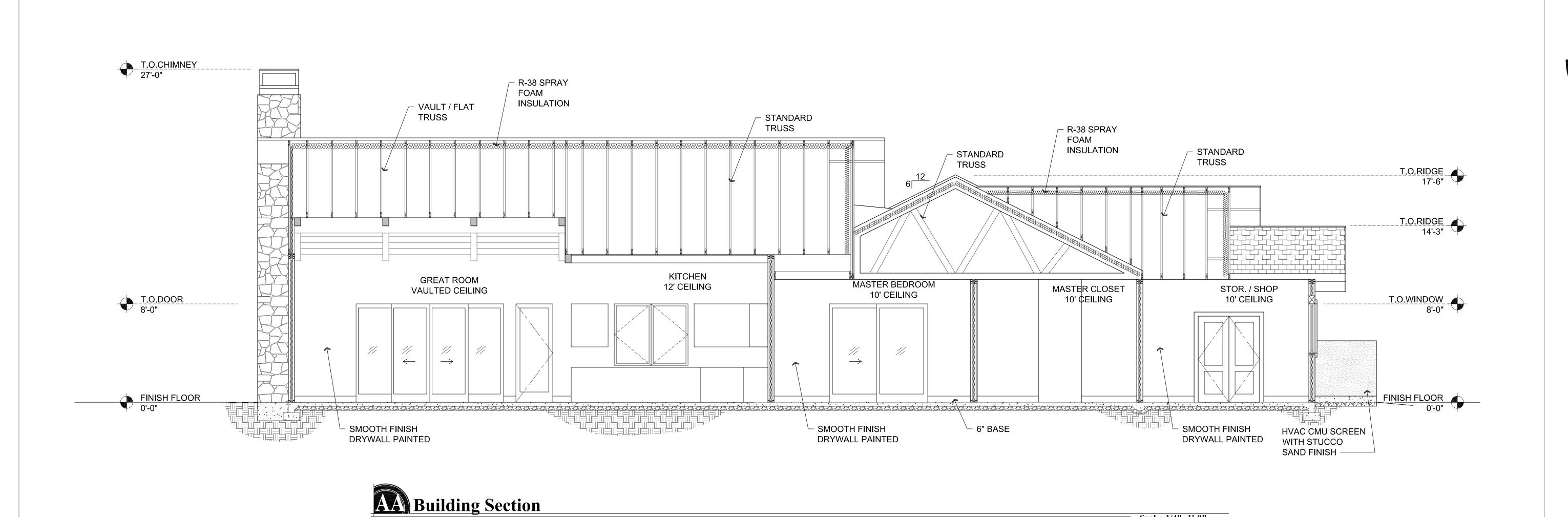
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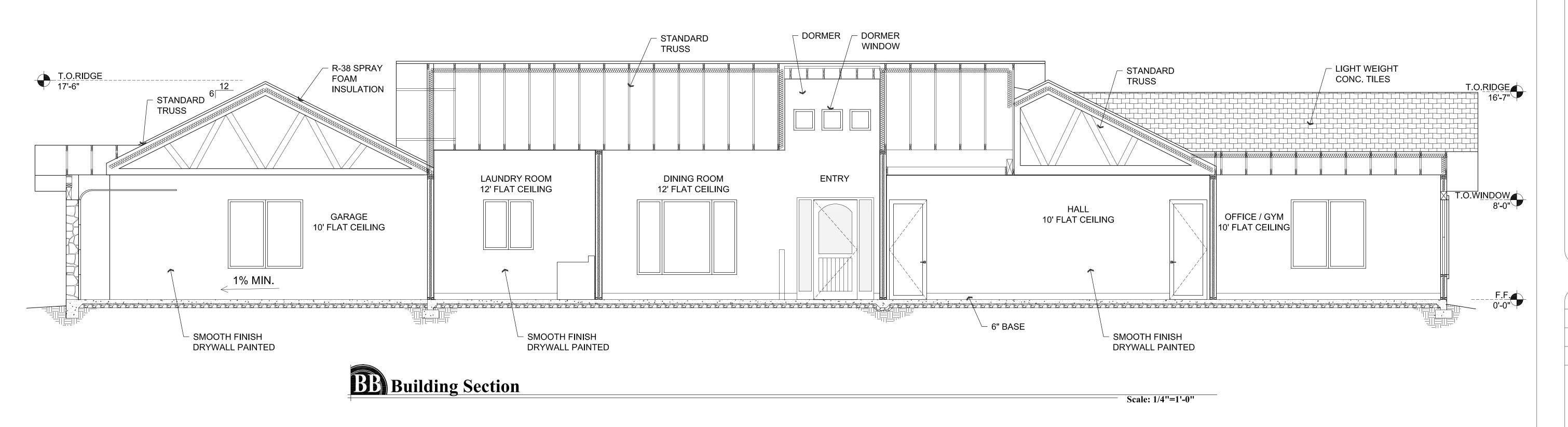
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A3.2





Scale: 1/4"=1'-0"

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DRAWING: Building Sections
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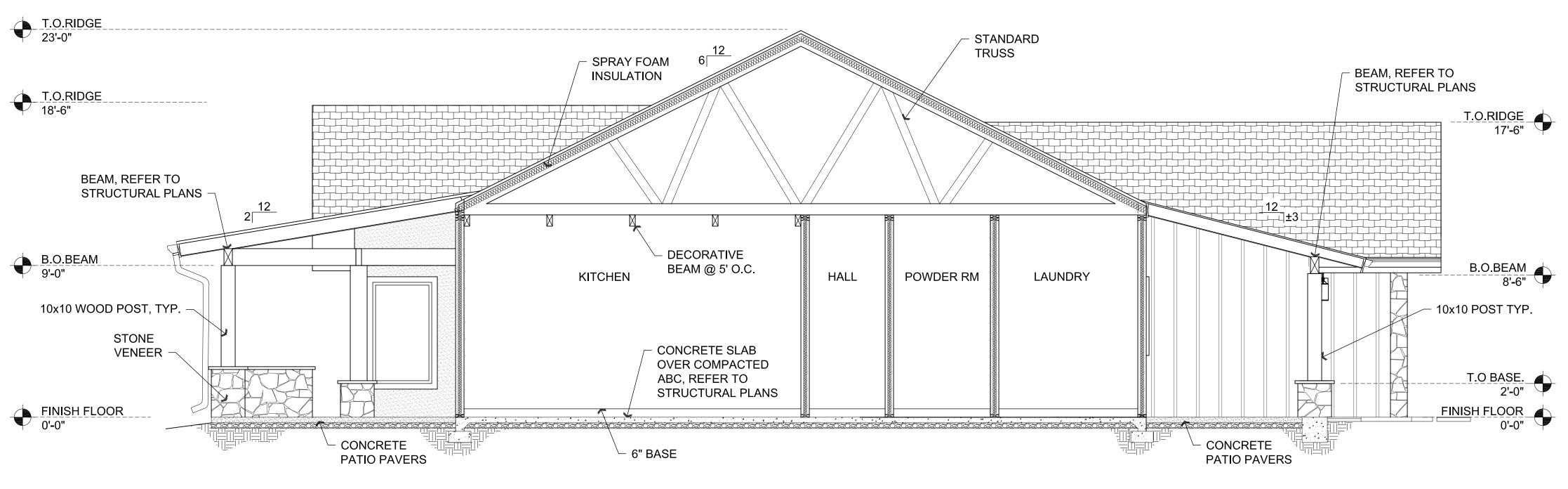
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Scale: 1/4"=1'-0"

Building Section

Scale: 1/4"=1'-0"

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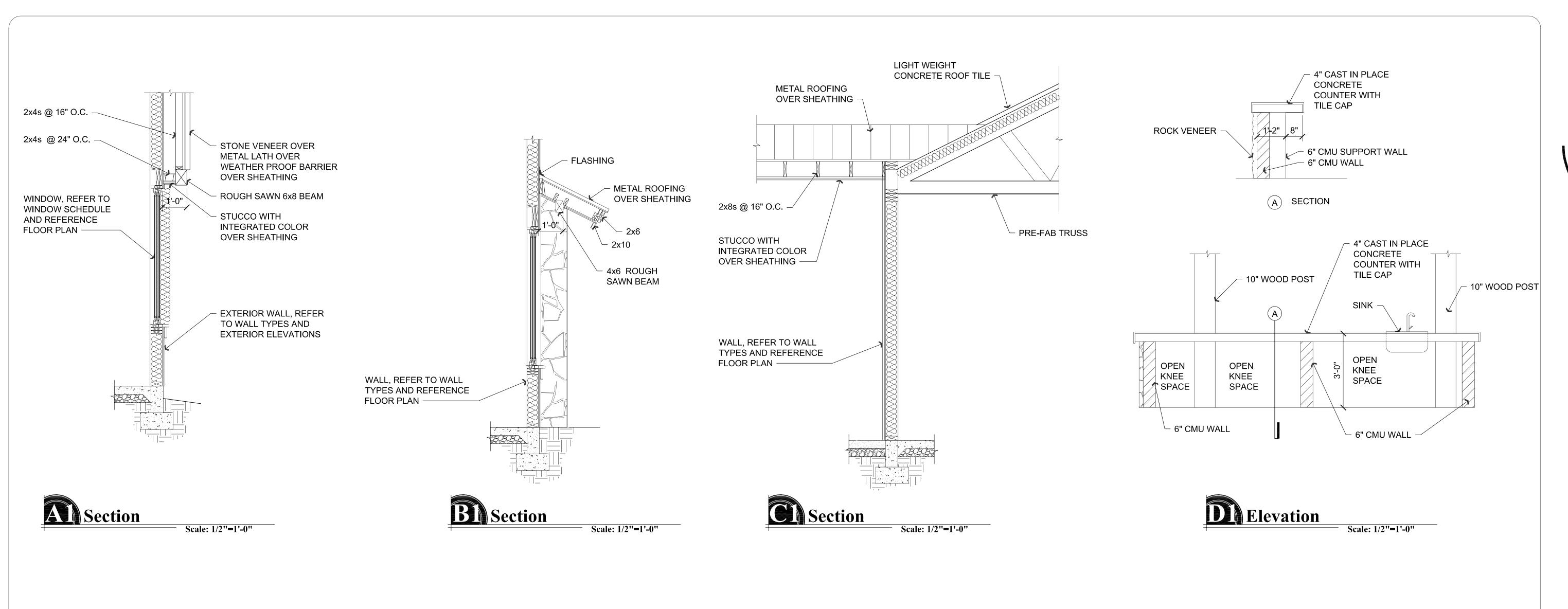
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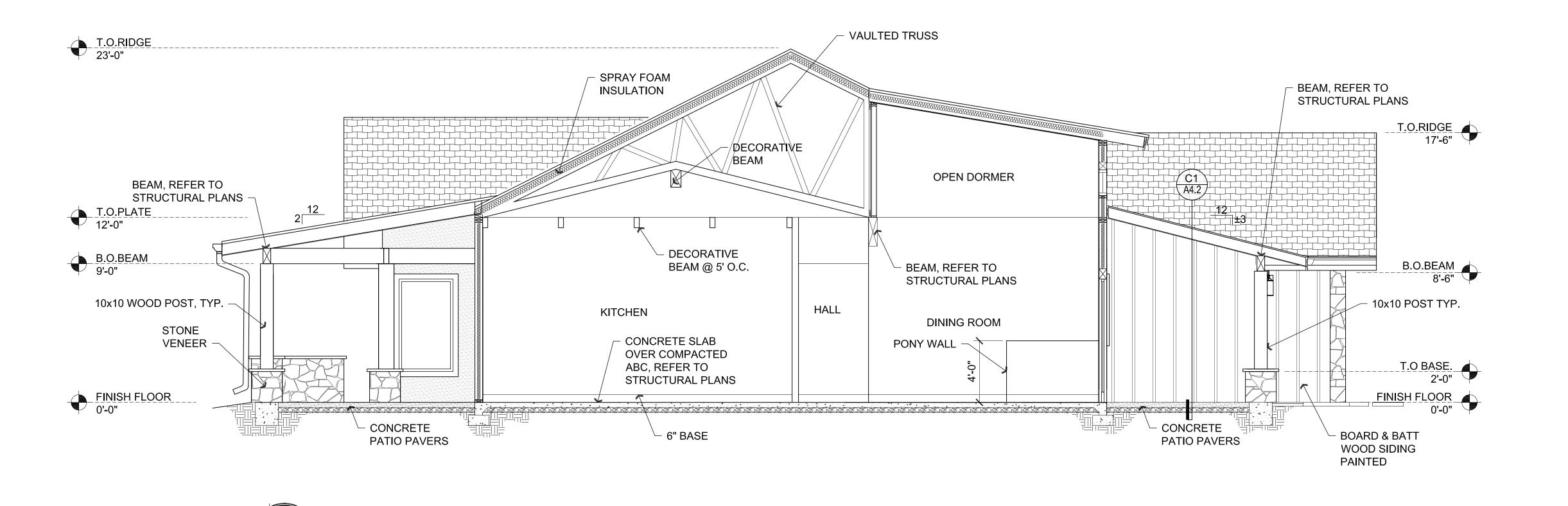
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SHEET





Scale: 1/4"=1'-0"

Building Section

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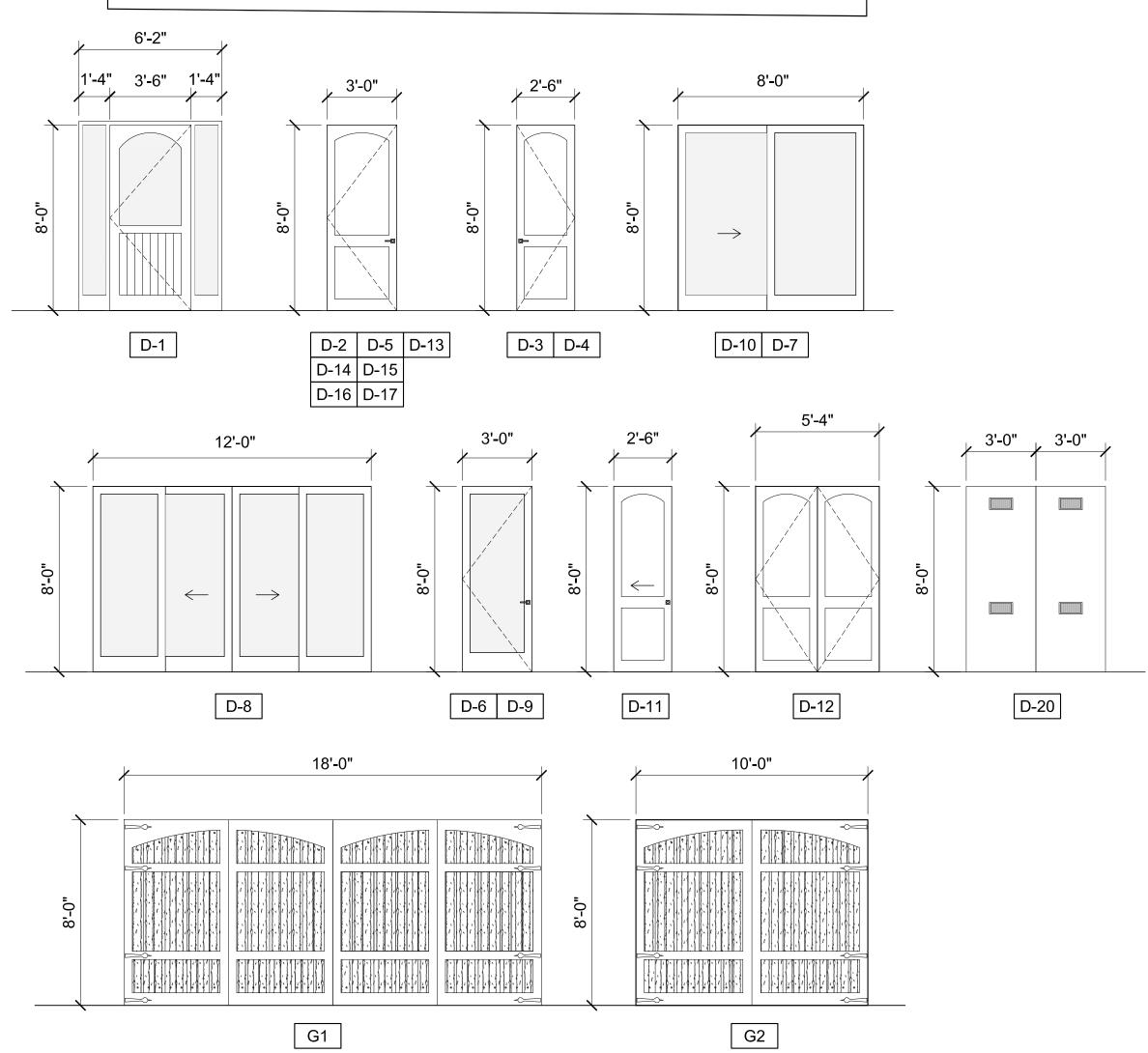
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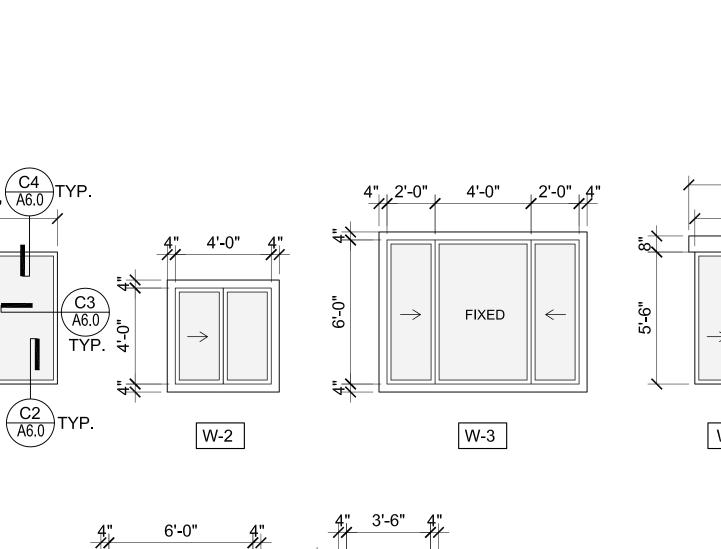
DOOR SCHEDULE									
DOOR	DESCRIPTION	SIZE	DOOR		FRAME				
MKD			MATERIAL	FINISH	MATERIAL	FINISH	HARDWARE TYPE	COMMENTS	
	NEW DOORS								
D-1	SINGLE DOOR W/ 2 SIDE LITES	6'-0" x 8'-0"	WOOD/GLASS	STAIN	WOOD	STAIN	BY OWNER	FRONT ENTRY	
D-2	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BEDROOM 1	
D-3	SINGLE DOOR	2'-6" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BATHROOM 1	
D-4	SINGLE DOOR	2'-6" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BATHROOM 2	
D-5	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BEDROOM 2	
D-6	SINGLE DOOR W/ GLASS	3'-0" x 8'-0"	WOOD/GLASS	PAINT	WOOD	PAINT	BY OWNER	OFFICE / GYM	
D-7	SLIDING DOOR	8'-0" x 8'-0"	ALUM/GLASS	-	-	-	BY OWNER	OFFICE / GYM	
D-8	SLIDING DOORS	12'-0" x 8'-0"	ALUM/GLASS	-	-	-	BY OWNER	GREAT ROOM	
D-9	SINGLE DOOR W/ GLASS	3'-0" x 8'-0"	WOOD/GLASS	PAINT	WOOD	PAINT	BY OWNER	KITCHEN	
D-10	SLIDING DOORS	8'-0" x 8'-0"	ALUM/GLASS	-	-	=	BY OWNER	MASTER BEDROOM	
D-11	POCKET DOOR	2'-6" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	TOILET ROOM	
D-12	DOUBLE DOOR	5'-4" x 8'-0"	FIBERGLASS	PAINT	WOOD	PAINT	BY OWNER	STORAGE / SHOP	
D-13	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	STORAGE / SHOP	
D-14	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	GARAGE 20 MINUTE FIRE RATED WITH SELF CLOSING HINGES AND SELF LATCHING	
D-15	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	LAUNDRY ROOM	
D-16	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	POWDER ROOM	
D-17	SINGLE DOOR	3'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	MASTER BEDROOM	
D-18	BI-PASS CLOSET DOOR	6'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BEDROOM 2	
D-19	BI-PASS CLOSET DOOR	6'-0" x 8'-0"	WOOD	PAINT	WOOD	PAINT	BY OWNER	BEDROOM 1	
D-20	DOUBLE DOOR	6'-0" x 8'-0"	FIBERGLASS	PAINT	WOOD	PAINT	BY OWNER	GARAGE WITH HIGH AND LOW 12"x6" VENTS	
G-1	GARAGE DOOR	18'-0" x 8'-0"	WOOD	STAIN	-	-	BY OWNER	GARAGE	
G-2	GARAGE DOOR	10'-0" x 8'-0"	WOOD	STAIN	-	-	BY OWNER	GARAGE	

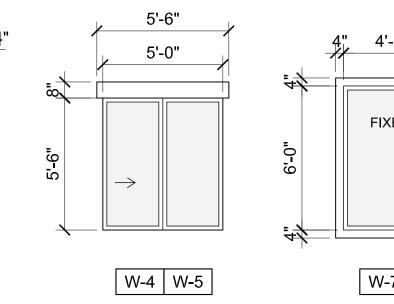
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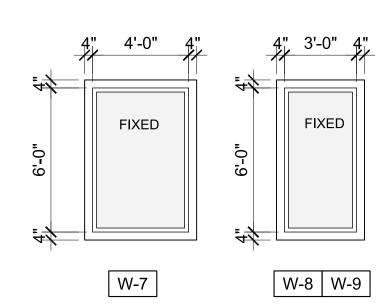
- 1. ALL GLAZING IN DOORS SHALL BE SAFETY GLAZING.
- 2. ALL GLAZING WITHIN 24" OF OPENINGS SHALL BE SAFETY GLASS.
- 3. IF A DOOR HAS A CLOSER, THEN THE SWEEP PERIOD OF THE CLOSER SHALL BE ADJUSTED SO THAT FROM AN OPEN POSITION OF 70 DEGREES, THE DOOR WILL TAKE AT LEAST 3 SECONDS TO MOVE TO A POINT 3" FROM THE LATCH, MEASURED TO THE LEADING EDGE OF THE DOOR.
- 4. ALL WINDOWS ARE TO HAVE A MAXIMUM U FACTOR OF .33.
- 5. EXTERIOR DOORS SHALL BE MIN. 1-3/4" THICK.
- 6. ALL DIMENSIONS SHALL BE VERIFIED IN FIELD BEFORE FABRICATION / ORDER.
- 7. FINISHES AND MANUFACTURERS MUST BE APPROVED BY OWNER.

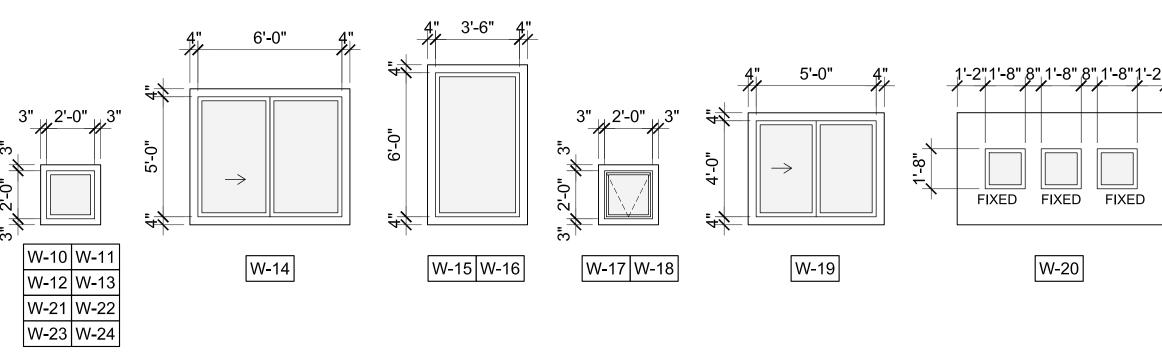












WINDOWS SCHEDULE

COMMENTS

BEDROOM - 1 MUST MEET EGRESS REQUIREMENTS

BEDROOM - 2 MUST MEET EGRESS REQUIREMENTS

FINISH

BRONZE

ESPRESSO

ESPRESSO

BRONZE

BRONZE

BRONZE

BRONZE

BRONZE

BRONZE

GARAGE

LAUNDRY ROOM

DINING ROOM

OFFICE / GYM

HALLWAY

HALLWAY

GREAT ROOM

GREAT ROOM

GREAT ROOM

GREAT ROOM

STORAGE / SHOP

GREAT ROOM

GREAT ROOM

GREAT ROOM

GREAT ROOM

DORMER

KITCHEN

OFFICE / GYM - EAST SIDE

MASTER BATH - TEMPERED

MASTER BATH - TEMPERED

MASTER BATH - TOILET - TEMPERED

MASTER BATH - SHOWER - TEMPERED

MATERIAL

WOOD / ALUM CLAD

FIBERGLASS

FIBERGLASS

WOOD / ALUM CLAD

size

6'-0" x 5'-6"

4'-0" x 4'-0"

8'-0" x 6'-0"

5'-0" x 5'-6"

5'-0" x 5'-6"

6'-0" x 5'-6"

4'-0" x 6'-0"

3'-0" x 6'-0"

3'-0" x 6'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

6'-0" x 5'-0"

3'-6" x 6'-0"

3'-6" x 6'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

5'-0" x 4'-0"

1'-8" x 1'-8"

2'-0" x 2'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

2'-0" x 2'-0"

NOTES: 1. ALL DIMENSIONS SHALL BE VERIFIED IN FIELD BEFORE FABRICATION / ORDER

2. FINISHES AND MANUFACTURERS MUST BE APPROVED BY OWNER



WINDOW

W-4

W-5

W-6

W-8

W-9

W-10

W-11

W-12

W-13

W-14

W-15

W-16 W-17

W-18

W-19

W-20 W-21

W-22 W-23

 \rightarrow

W-1 W-6

DESCRIPTION

ALUMINUM WINDOW / SLIDER

ALUMINUM WINDOW / FIXED

ALUMINUM WINDOW / SLIDER

ALUMINUM WINDOW / FIXED

ALUMINUM WINDOW / FIXED

ALUMINUM WINDOW / SLIDER

ALUMINUM WINDOW / FIXED

FIBERGLASS / AWNING

FIBERGLASS / AWNING

ALUMINUM WINDOW / XOX SLIDER

NEW WINDOWS

Scale: 1/4"=1'-0"

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P 928-443-5812 P.O. Box
F 928-443-5815 Prescott,
email: wakaarchitect@gmail.o

Estabrook Residence 9185 N. American Ranch Road Prescott, AZ 86305

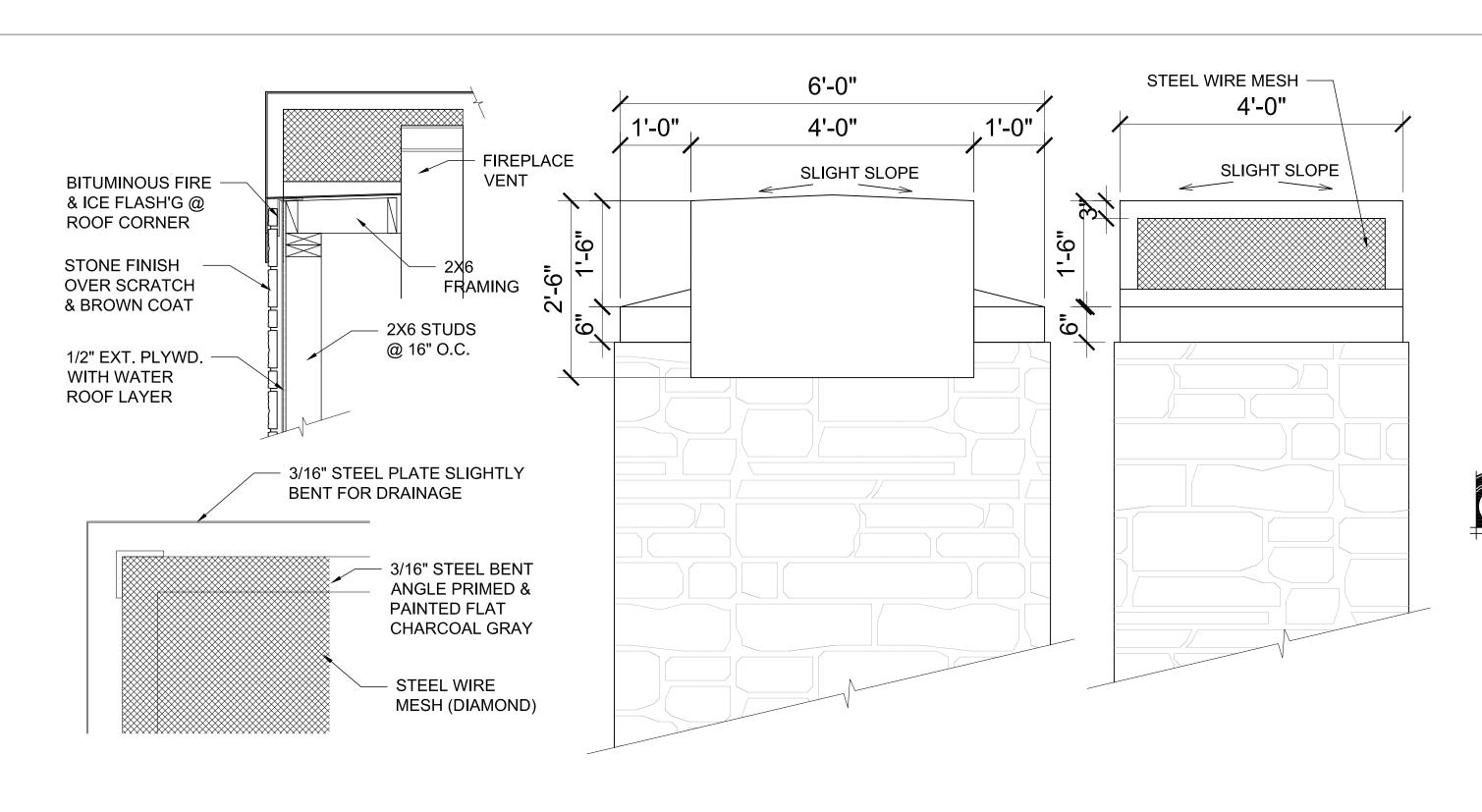
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L.O.

CHECKED BY
W.A.K.

DATE
June 14th, 2023

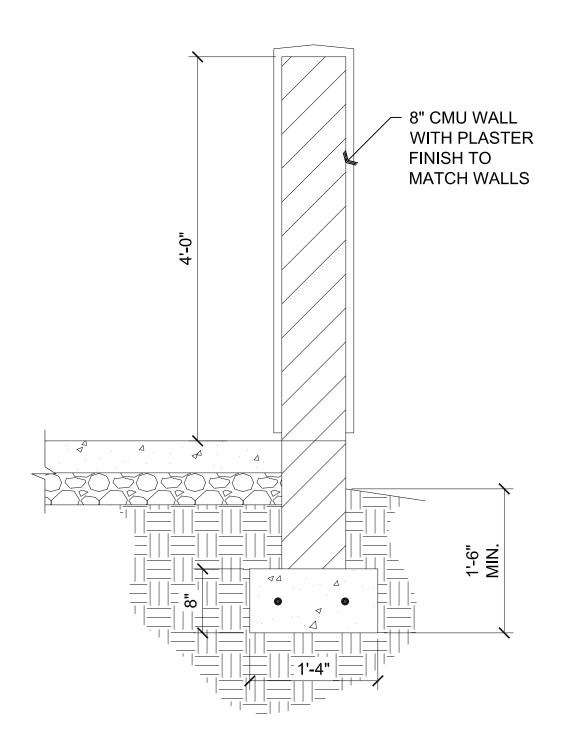
JOB NO. **792** SHEET

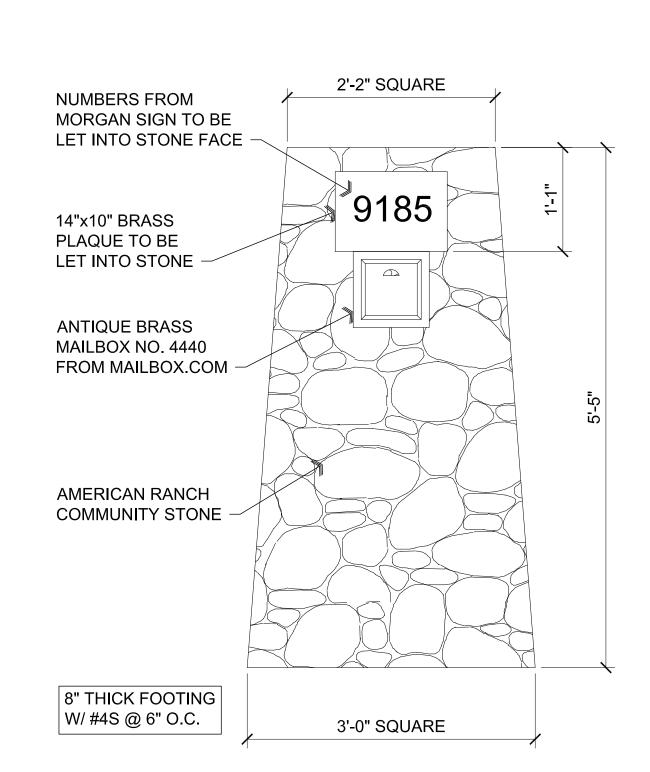
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Chimney Details

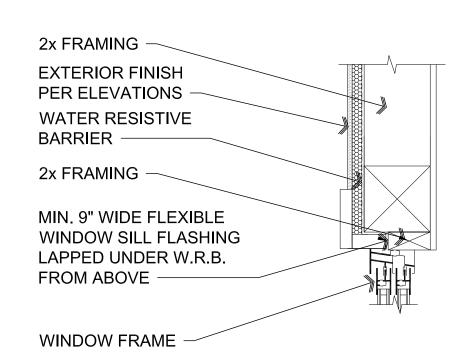
Scale: 3/4"=1'-0"



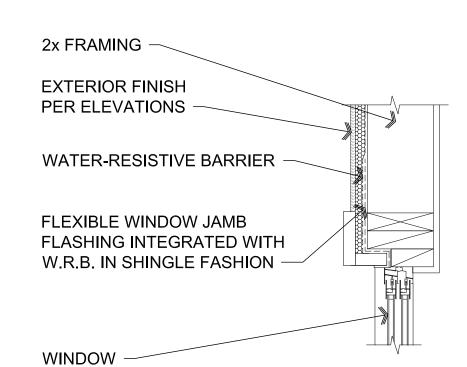




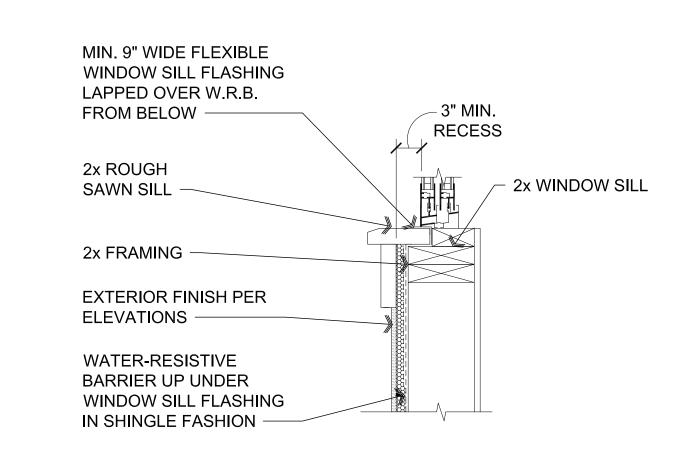


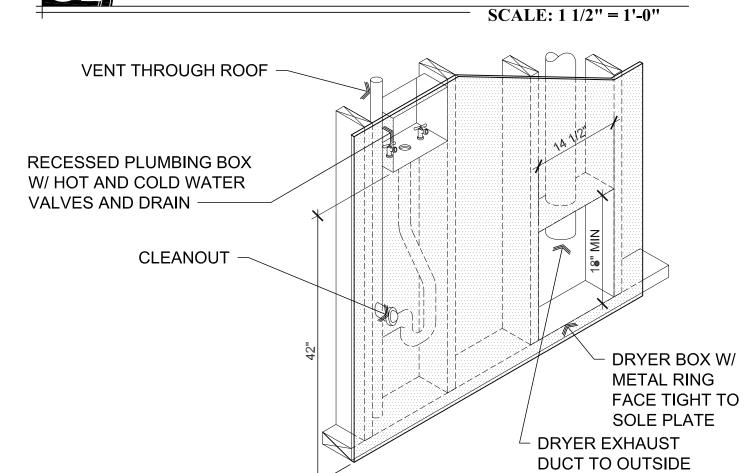


Recessed Window Head SCALE: 1 1/2" = 1'-0"



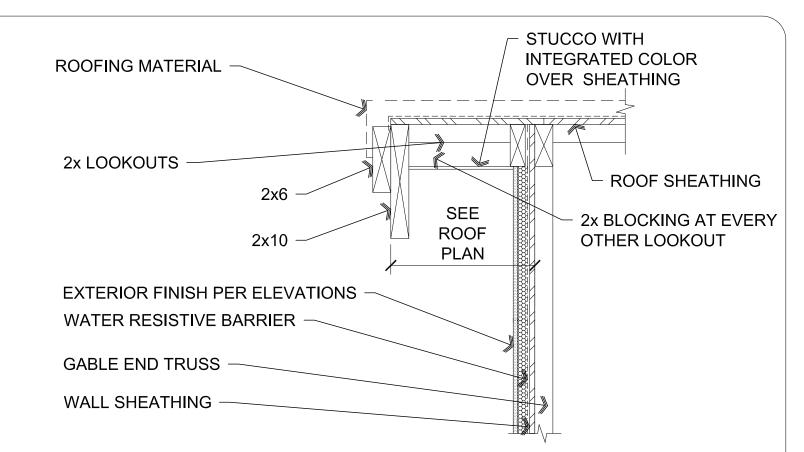
Recessed Window Jamb SCALE: 1 1/2" = 1'-0"



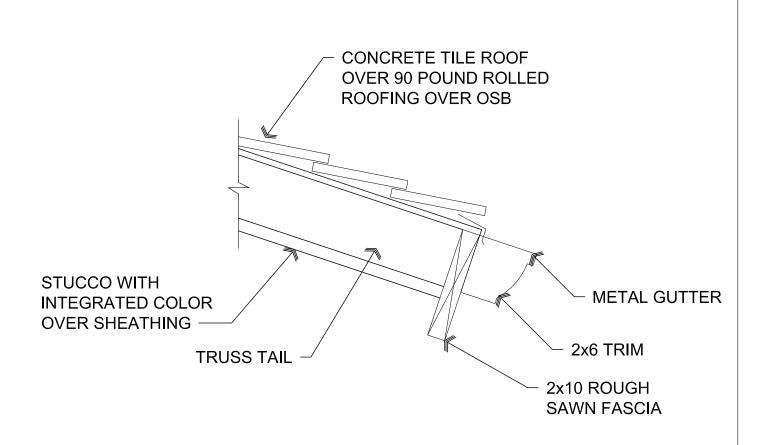


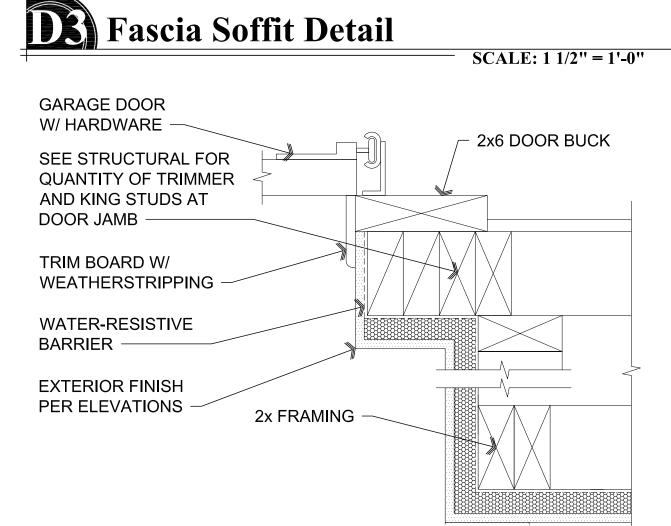


Recessed Window Sill

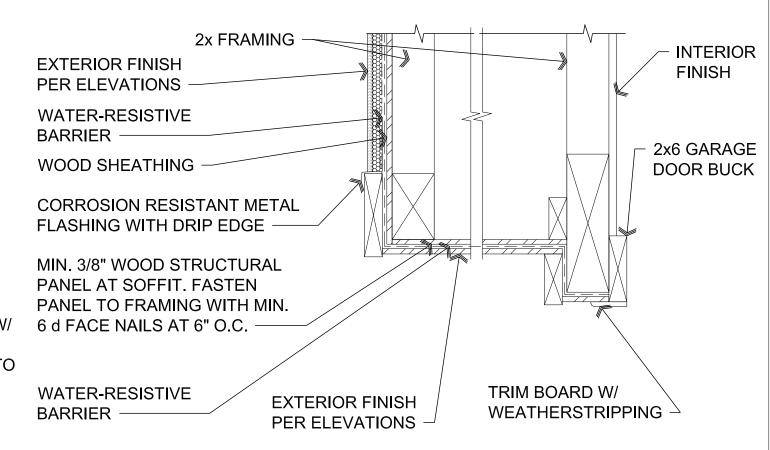


Barge Exposed Overhang











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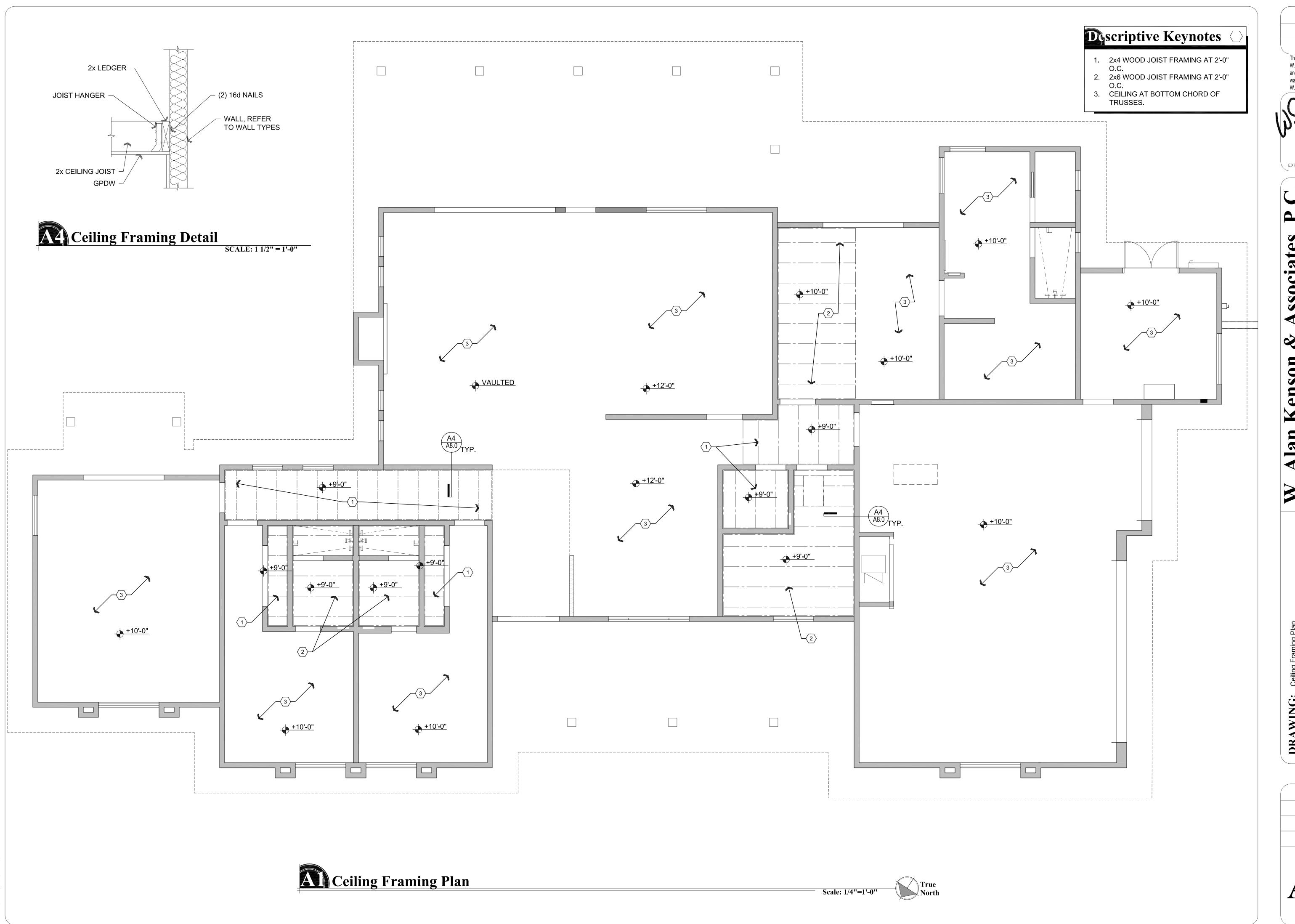
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A7.0



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P 928-443-5812 Prescott, AZ 86304 email: wakaarchitect@gmail.com

DRAWING: Ceiling F
PROJECT: Estabroo

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792

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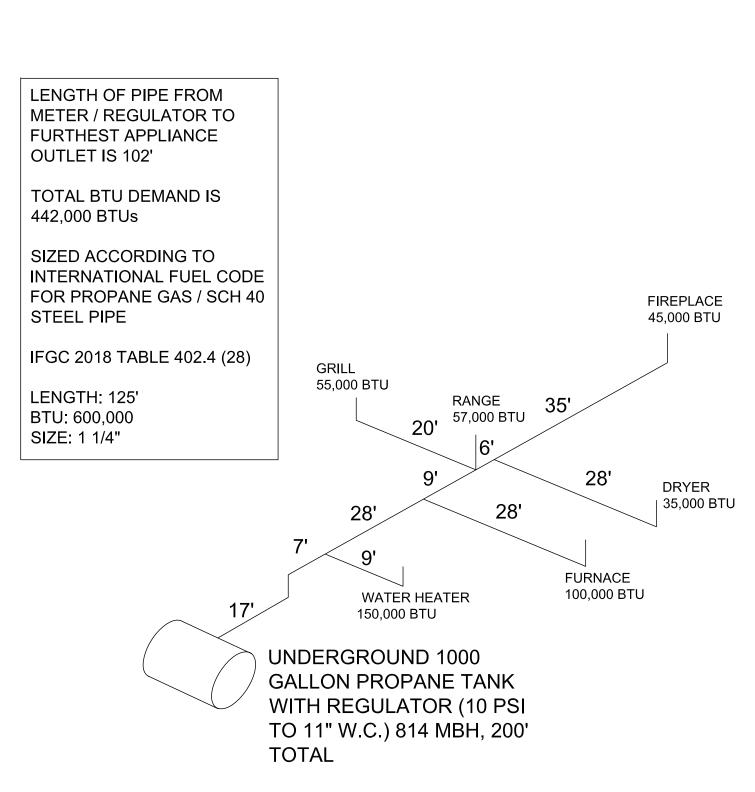
Exture Unit Calculations							
TYPE	COUNT	НОТ	COLD	COMBINED	TOTAL		
FULL BATH GROUP	3	1.5	2.7	3.6	10.8		
KITCHEN GROUP	1	1.9	1	2.5	2.5		
LAUNDRY GROUP	1	1.8	1.8	2.5	2.5		
HOSE BIBB	5	0	2.5	2.5	12.5		
LAVATORY	1	.5	.5	.7	.7		
WATER CLOSET	1	0	2.2	2.2	2.2		
SINK	3	1	1	2.4	7.2		
				TOTAL	38.4		

38.4 WATER SUPPLY FIXTURE UNITS = 26.3 GALLONS PER MINUTE 2" WATER LINE PROPOSED

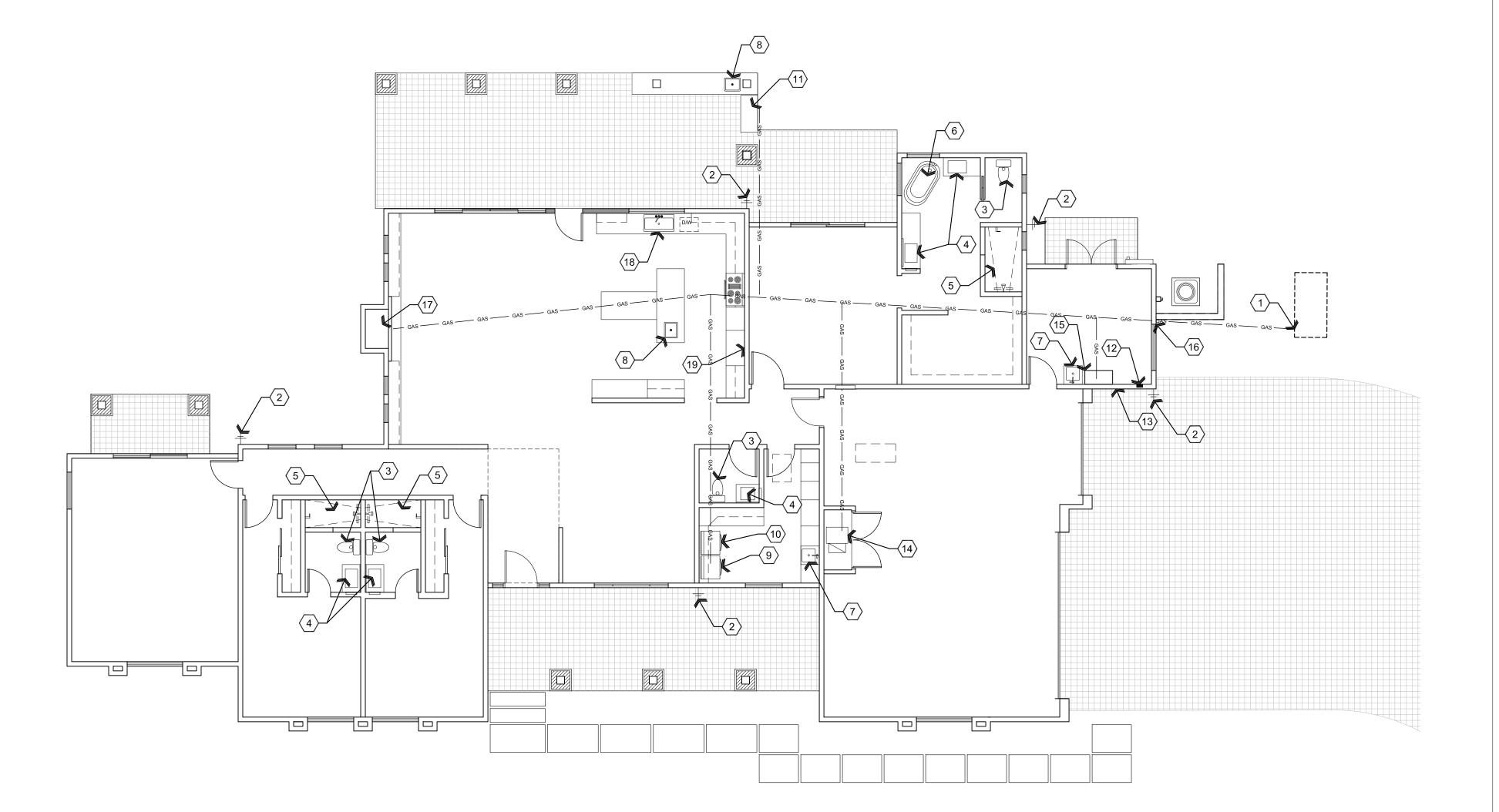
Discriptive Keynotes \bigcirc

- 1. PROPANE SHUT OFF VALVE.
- 2. FROST PROOF HOSE BIBB.
- 3. TOILET AS SELECTED BY OWNER
- 4. INTEGRAL LAVATORY AS SELECTED BY OWNER.
- 5. SHOWER AS SELECTED BY OWNER.
- 6. BATHTUB AS SELECTED BY OWNER.
- 7. UTILITY SINK.
- 8. SINGLE BOWL PREP SINK.
- 9. WASHING MACHINE.
- 10. PROPANE CLOTHES DRYER.11. PROPANE GRILL LOCATION.
- 12. FIRE SPRINKLER RISER WITH SHUT OFF VALVE.
- 13. MAIN WATER SHUT OFF VALVE.
- 14. HVAC UNIT.
- 15. PROPANE INSTANTANEOUS WATER HEATER.
 16. LPG RISER FROM BELOW GRADE. PENETRATE EXTERIOR WALL AT +1'-6" ABOVE GRADE AND ROUTE LPG PIPING WITHIN WALL CAVITY OR
- ABOVE CEILING.

 17. PROPANE FIRED LOG FIREPLACE.
- 18. KITCHEN SINK AS SELECTED BY OWNER.
- 19. REFRIGERATOR LOCATION WITH WATER LINE FOR ICE MAKER.







Plumbing Plan

Scale: 1/8"=1'-0"

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Estabrook Residence 9185 N. American Ranch Road

ROJECT: Estabroo

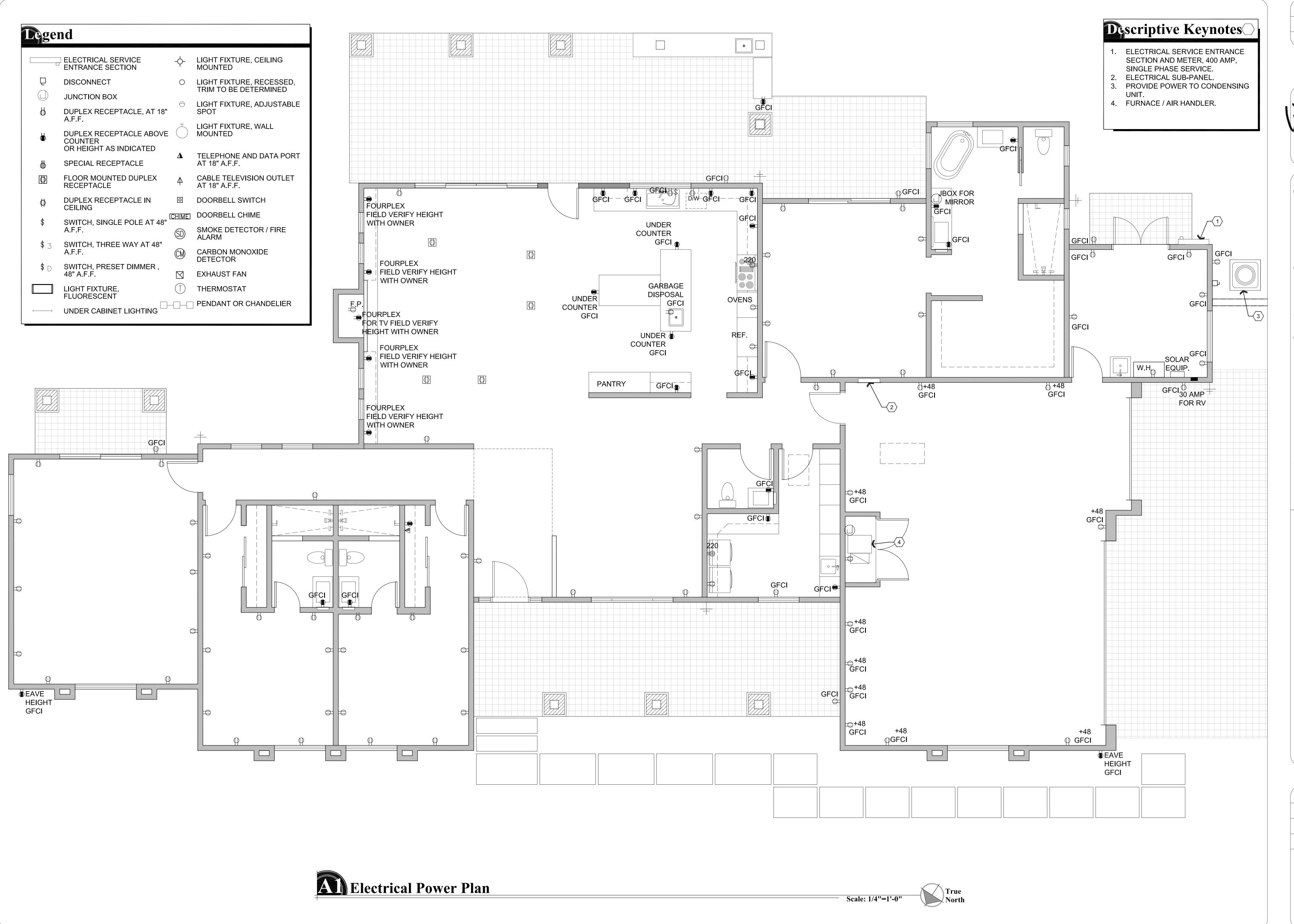
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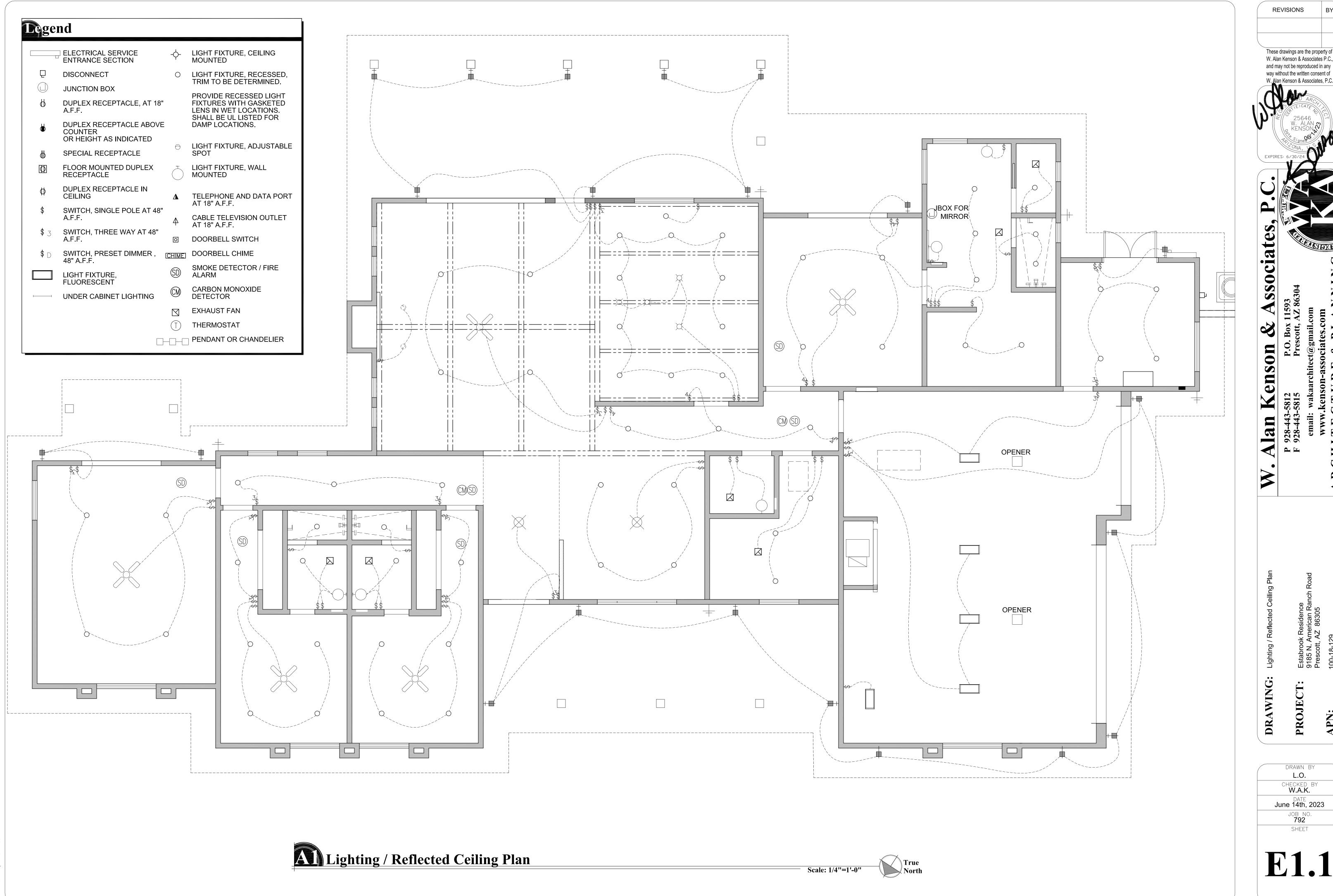
DATE
June 14th 2023

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June 14th, 2023

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CHECKED BY W.A.K. June 14th, 2023

5000 VA

1,500 VA FOR EACH 20 AMP BRANCH CIRCUIT REQUIRED PER 210.1(C)(1) IN EACH KITCHEN,

PANTRY, BREAKFAST ROOM, DINING ROOM OR SIMILAR AREAS.

NUMBER OF SMALL APPLIANCE BRANCH CIRCUITS = $2 \times 1,500 \text{ VA} = 3,000 \text{ VA}$

220.52(B) LAUNDRY LOAD

NUMBER OF LAUNDRY BRANCH CIRCUITS = 1 x 1,500 VA = 1,500 VA

15,633VA TOTAL GENERAL LIGHTING AND APPLIANCE LOAD =

LIGHTING LOAD FEEDER DEMAND FACTORS

3,000 VA FIRST 3,000 OR LESS AT 100% = 4,422 VA FROM 3,001 TO 120,000 AT 35% = REMAINDER OVER 120,000 AT 25% =

TOTAL LIGHTING DEMAND LOAD = 7,422 VA

ELECTRIC CLOTHES DRYERS

ELECTRIC RANGES ELECTRIC RANGE NAMEPLATE KW = 22.5 KW WALL MOUNTED OVEN NAMEPLATE KW = 0 KW ELECTRIC COOKTOP NAMEPLATE KW = 0 KW

> 14,625 VA ELECTRIC COOKING DEMAND LOAD AT 65% =

220.51 LARGEST OF FIXED ELECTRIC SPACE HEATING LOADS OR A/C LOAD 220.21

A/C #1 = 33.0 MCA AT 208-230V

7,360 VA TOTAL NON-COINCIDENT LOAD =

220.53 APPLIANCE LOAD

QUANTITY DESCRIPTION VA(WATTS) **DISHWASHER** 1,800

MICROWAVE 1,000 75 PADDLE FANS 725 REFRIGERATORS

> 3, 469 VA TOTAL CONNECTED APPLIANCE LOAD AT 75% =

MOTOR LOADS AT 120 VOLTS

TOTAL MOTOR LOAD =

53,509 VA TOTAL CALCULATED DEMAND LOAD IN VOLT-AMPERES =

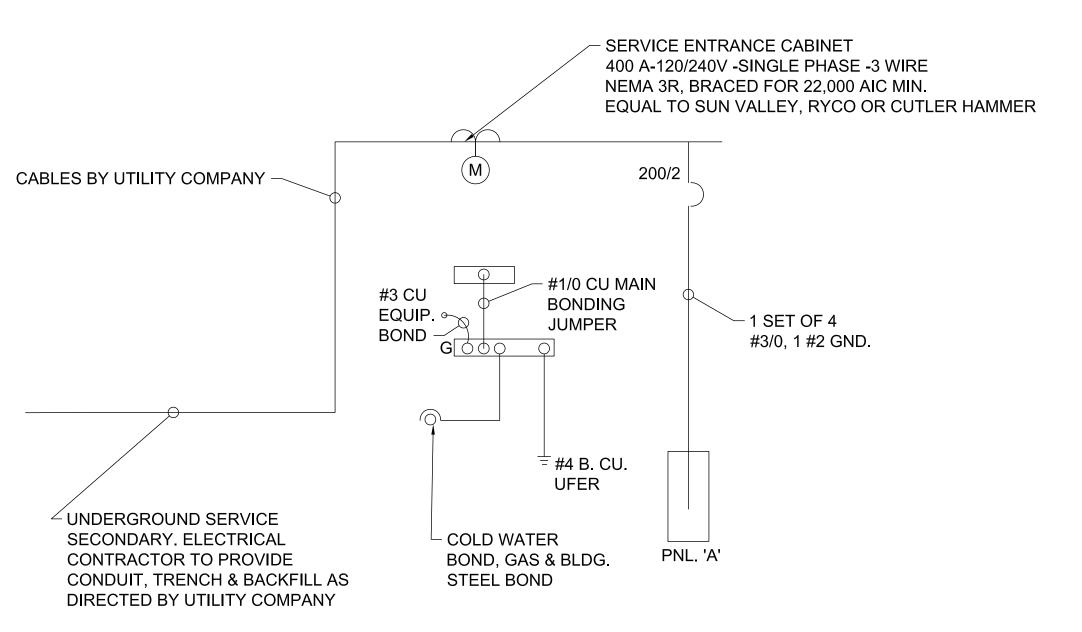
TOTAL CALCULATED DEMAND LOAD IN AMPS AT 1 PHASE 3 WIRE, 120/240 VOLTS = MINIMUM SERVICE REQUIRED = SERVICE SIZE REQUESTED =

223 AMPS 300 AMPS 400 AMPS

<u>0 VA</u>

Electric Load Calculations

Scale: N.T.S.





Note: Licensed Electrician to verify all wire sizes

General Electrical Notes:

- 1. A MINIMUM OF TWO 20-AMPERE RATED BRANCH CIRCUITS SHALL BE PROVIDED FOR RECEPTACLES LOCATED IN THE KITCHEN, PANTRY, BREAKFAST, AND DINING AREAS. AN ADDITIONAL 20 AMPERE RATED BRANCH CIRCUIT SHALL BE PROVIDED TO THE LAUNDRY AND A SEPARATE 20 AMPERE RATED BRANCH CIRCUIT SHALL BE PROVIDED FOR BATHROOM RECEPTACLES.
- 2. ALL BRANCH CIRCUITS THAT SUPPLY 120-VOLT, SINGLE-PHASE, 15- AND 20-AMPERE OUTLETS INSTALLED IN FAMILY ROOMS, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUNROOMS, RECREATIONS ROOMS, CLOSETS, HALLWAYS AND SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A COMBINATION TYPE ARC-FAULT CIRCUIT INTERRUPTER INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT.
- 3. IN EVERY KITCHEN, FAMILY ROOM, DINING ROOM, LIVING ROOM, PARLOR, LIBRARY, DEN, SUN ROOM, BEDROOM, RECREATION ROOM, OR SIMILAR ROOM OR AREA OF DWELLING UNITS, RECEPTACLES SHALL BE INSTALLED SO THAT NO POINT ALONG THE FLOOR LINE IN ANY WALL SPACE IS MORE THAT 6 FEET MEASURED HORIZONTALLY, FROM AN OUTLET IN THAT SPACE, INCLUDING ANY WALL SPACE 2 FEET OR MORE IN WIDTH.
- 4. IN KITCHEN AND DINING ROOMS, AT LEAST ONE RECEPTACLE OUTLET SHALL BE INSTALLED AT EACH ISLAND OR PENINSULAR COUNTER SPACE WITH A LONG DIMENSION OF 24 INCHES OR GREATER AND A SHORT DIMENSION OF 12 INCHES.
- 5. IN KITCHEN AND DINING ROOMS, AT LEAST ONE RECEPTACLE OUTLET SHALL BE INSTALLED AT EACH WALL COUNTER SPACE 12 INCHES OR WIDER SO THAT NO POINT ALONG THE WALL IS MORE THAN 24 INCHES FROM A RECEPTACLE OUTLET AND SHALL BE GFCI PROTECTED.
- 6. PROVIDE AT LEAST ONE WEATHERPROOF RECEPTACLE OUTLET, NOT MORE THAT 6 FEET 6 INCHES ABOVE GRADE AND GFCI PROTECTED, AT THE FRONT AND BACK OF EACH DWELLING. ALL RECEPTACLES INSTALLED OUTDOORS MUST BE GFCI PROTECTED.
- 7. PROVIDE AT LEAST (1) ONE RECEPTACLE OUTLET IN HALLWAYS 10 FEET OR MORE IN LENGTH.
- 8. A 125 VOLT, SINGLE PHASE, 15 OR 20 AMPERE RATED RECEPTACLE OUTLET SHALL BE INSTALLED AT AN ACCESSIBLE LOCATION FOR THE SERVICING OF HEATING, AIR CONDITIONING AND REFRIGERATION EQUIPMENT. THE RECEPTACLE SHALL BE LOCATED ON THE SAME LEVEL AND WITHIN 25 FEET OF THE EQUIPMENT.
- 9. ALL 125 VOLT, SINGLE PHASE, 15 AND 20 AMPERE RECEPTACLES IN THE FOLLOWING LOCATIONS SHALL BE GFCI PROTECTED: BATHROOMS, GARAGES, UNFINISHED ACCESSORY BUILDINGS, CRAWL SPACES, UNFINISHED BASEMENTS, BAR SINKS (WITHIN 6 FEET) AND LAUNDRY ROOM SINKS (WITHIN
- 10. PROVIDE AT LEAST (1) ONE WALL MOUNTED SWITCH CONTROLLED LIGHTING OUTLET IN EVERY HABITABLE ROOM AND BATHROOM.
- 11. PROVIDE A LIGHTING OUTLET ON THE EXTERIOR SIDE OF ALL EXITS/ENTRANCES.
- 12. A RECEPTACLE SHALL NOT BE INSTALLED WITHIN A BATHTUB OR SHOWER
- 13. FIXTURES, FITTINGS, BOXES AND RECEPTACLES LOCATED IN DAMP OR WET LOCATIONS SHALL BE "LISTED" TO BE SUITABLE FOR SUCH LOCATION.
- 14. PROVIDE INTERCONNECTED SMOKE ALARMS IN EACH SLEEPING ROOM, IMMEDIATELY OUTSIDE EACH SLEEPING ROOM, ON EACH ADDITIONAL STORY INCLUDING BASEMENTS, AND IN THE HALLWAY. SMOKE ALARMS SHALL BE HARD WIRED WITH BATTERY BACKUP.
- 15. PROVIDE A GROUNDING ELECTRODE SYSTEM. PROVIDE BONDING TO THE INTERIOR WATER PIPING AND ABOVE GROUND PORTION OF GAS PIPING SYSTEM.
- 16. EXTERIOR LIGHTING SHALL BE DARK SKY COMPLIANT.

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E1.2

JOB NO **792**

<u>CODE</u>: Comply with 2018 I.R.C. & 2018 I.B.C. and all current adopted municipal amendments.

C (Site Specific)

WIND DESIGN SPEED: 115 m.p.h., (Ultimate)

<u>SEISMIC DESIGN CATEGORY</u>:

<u>SEISMIC SOIL SITE CLASS:</u> WEATHERING: Moderate

FROST LINE DEPTH: 18" below Finish Grade SNOW LOAD: 30 PSF

See FIRM, FEMA, NFIP, FBFM Maps FLOOD HAZARDS:

SUPERIMPOSED DEAD & LIVE DESIGN LOADS: See Framing Plans for information.

MECHANICAL EQUIPMENT LOADS: See Mechanical Plans for information.

SHOP DRAWINGS:

<u>WIND EXPOSURE</u>:

1. The structural shop drawing review is intended to help the Engineer verify his design concept. It is the Contractor's responsibility to check his own shop drawings and those of his Subcontractors.

2. The structural shop drawings will be returned for resubmittal if not checked by Contractor or a cursory review shows major errors which should have been found by the Contractor's checking.

3. CONCRETE MIX DESIGNS: When required, submittals shall be prepared or certified to conform to ACI Code by an independent testing laboratory prior to submitting to architect. Each separate mix design shall be included with a cover letter indicating all locations on the project where the mix will be used.

4. When required by the Building Department, for the categories listed below, shop drawings and calculations shall be submitted for structural review. Provide drawings and calculations to S. E. Consultants, Inc. for review prior to submitting for plan review.

- 1. Wood trusses. 2. Glulam beams.
- 3. Spliced reinforcing.

5. Any resubmittal of a detail sheet with added information shall be accompanied by location plan identifying the members involved, and clouding around added information.

6. Dimensions will not be checked. Dimension checking and checking of design changes proposed by Contractor without prior consultation with the Engineer shall be checked only if the Contractor wishes them to be checked at his cost.

7. Any Engineering submitted for review shall be appropriately sealed. Full responsibility of such Engineering rests with the person sealing the design.

DEFERRED SUBMITTALS

Drawings and calculations shall be sealed by an Arizona Registered Engineer and must be forwarded to the Municipal Inspector with a notation from the Engineer of Record that they have been reviewed and approved for general conformance with the original design of the structure. The deferred submittal items shall not be installed until their design and submittal documents have been approved by the Building Official. These documents shall be submitted to the Development Service Department Field Inspector prior to installation.

The following items shall have deferred submittals:

New Pre-Engineered Wood Trusses

2. Engineering and Construction details for any Existing Truss modifications when occurs.

STRUCTURAL INSPECTION:

It is the Contractor's responsibility to inspect all structural work for conformance with the contract documents. Any structural inspection provided by others does not relieve him of this responsibility. Any structural deviations from the contract documents that are found at a later date and are declared to be significant by the Structural Engineer shall be corrected by the Contractor with all dispatch. The Structural Inspector is not authorized to direct or approve any changes from the contract documents. If the Contractor wishes to question the Structural Inspector's interpretation of the contract documents, he may do so directly with the Architect or the Structural Engineer.

The Structural Inspector is not authorized to stop or delay the work. If the contractor elects to continue with a certain work after being notified by the Structural Inspector that such work is unacceptable, he does so at his own responsibility and risks correcting the work at a less opportune time.

The Structural Inspector is not inspecting for OSHA compliance and temporary construction, such as bracing. The Contractor is responsible for providing adequate facilities for the Structural Inspector, to allow him to perform his work safely and efficiently.

SPECIAL INSPECTION:

Special inspection by a Municipal approved special inspectors is required for the following types of work in conformance with the current adopted International Building Code (I.B.C.). All Inspections are to be performed on a continuous basis, unless noted otherwise.

The Special Inspection(s) noted below as APPLICABLE are required for the following Construction Categories: Those Categories noted N / A are not applicable to this Project.

INSPECTION OF STEEL FABRICATORS:

A. All Structural Steel load—bearing components and assemblies performed on the premises of a fabricator's shop if not done by a Municipal approved fabricator.

2. N / A : STEEL CONSTRUCTION:

Required for all Steel Construction including shop welding if welding is <u>not</u> performed by a Municipal approved fabricator.

Field welding of Structural Steel components. All structural welding is to be provided with continuous special inspections except where periodic special

inspections are allowed by the I.B.C. Code. Special inspections are not required when the work is done on the

premises of a Municipal approved steel fabricator.

FIELD WELDING: of the following, 3. N / A : Structural Steel Components (see above)

Rebar splice welding

4. N / A : CONCRETE CONSTRUCTION:

Steel reinforcing Concrete of 3,000 psi or greater strngth

5. N / A : MASONRY CONSTRUCTION:

All Engineered retaining walls.

6. N / A : WOOD CONSTRUCTION:

7. N / A : PILE / PIER FOUNDATIONS:

POST-INSTALLED EPOXIED & MECHANICAL ANCHORS / EPOXIED REINFORCING DOWELS (Per Structural Details) 8. Applicable

9. N / A : GEO-TECHNICAL INSPECTIONS: To be performed by Civil or Soils Engineer as required per separate S.I. Form.

FOUNDATIONS:

Soil Report by:Engineering & Testing Consultants Inc. 417 N. Arizona Ave. Prescott, Arizona 86301 928.778.9001 Project Report #: 12074

Dated: 01/10/2023

Basis for Foundation design is 1,500 psf. allowable soil bearing capacity. Per soils report dated 01/10/23 and addendum dated 06/09/23 'In all areas where the building pad has been completed with 36" or more of imported, compacted, granular, engineered fill, footings may be seated at a min. embedment depth of 18" below lowest, adjacent finished grade, bearing in the controlled compacted fill.' Special attention should be given to final grades and landscaping improvements to ensure efficient draining away from foundations and slabs.

<u>CONCRETE</u>

Shall meet all the requirements of ACI 301-15 with Type II cement. Minimum 28 day strength 3,000 p.s.i., except as follows:

Sidewalks curbs, and gutters,

.....2,500 p.s.i. slabs on grade.....

Wall foundations, grade beams, slabs on grade......2,500 p.s.i.

No admixtures without approval. Admixtures containing chlorides shall not be used. Concrete shall not be in contact with aluminum.

Fly ash shall not be used unless approved by Engineer of Record.

Mechanically vibrate all concrete when placed, except that slabs on grade need be vibrated only around embedded items. Slump 4 inches for slabs not on grade and 5 inches for other concrete. Do not tamp slabs. Use roller bug, vibrating screed or bull float to finish. Do not add water to concrete at site.

All reinforcing, including dowels and anchor bolts, shall be securely tied in location before placing concrete or grout. Dowels will not be allowed to be "stabbed" in.

Cure uncovered slabs on grade and job cast precast panels with polyethylene for 5 days. Tape joints with 6 inch laps and cover with sand. Curing compound for other work shall be compatible with applied finish, conform to ASTM C-309 and shall be clear on uncovered structure and white pigmented on covered structure. Apply at a rate sufficient to retain moisture, but not less than 1 gallon per 200 square feet.

Cast slabs on grade in alternate sections, unless permanent forms are used. Wait 48 hours between all adjacent concrete castings. Revibrate tops of columns and caissons soon after placing concrete to close plastic shrinkage cracks. Do not place concrete in lengths exceeding 100 feet.

The Contractor shall fix all cracks and displacements larger than 1/16" up to the project completion.

Minimum strength for removal of bottom forms and shoring shall be 75% of specified strength at 28 days.

All concrete which during the life of the structure will be subjected to freezing temperatures while wet, shall have a water cement ratio not exceeding 0.53 by weight and shall contain entrained air as per ACI 301. Such concrete shall include exterior slabs, perimeter foundations, exterior curbs and autters, etc.

When span L exceeds 10'-0", camber up all concrete beams and slabs L/400 at midspan. Camber up all overhangs L3"00 at edge of cantilever. Record cambers at undersides of structure immediately before and after reshoring and immediately after de—shoring.

An anticipated deflections of steel floor beams and girders under weight of wet concrete are L/400. Set screeds to compensate for the deflections and any construction deviations within specified tolerances, so that the finished floor is level. Allow 1/2 inch additional concrete in the bid for leveling.

HIGH-RANGE WATER REDUCING ADMIXTURE (SUPER PLASTICIZER):

The Contractor shall use super plasticizer admix in locations indicated on drawings and shall consider its use where congestion of rebar is likely to cause rock pockets. The cement for the mix shall be Type II. The rate of placing such concrete shall be reduced or the form strength shall be increased to safely resist increased pressure against forms.

The use of the admix shall be in strict accordance with the manufacturer's specifications and ACI recommendations. Do not use with colored concrete.

Maximum slump may be increased over specified slump but shall not exceed 8".

CONCRETE REINFORCING:

ASTM A-615 Grade 60 except as follows:

#2 bars..... Grade 40

#7 and larger bars to be welded...... A-706

Wire mesh, flat sheets..... A-185

Welded anchors....... Grade 40, chemical analysis limited per AWS spec for weld without preheat. Also see "Welding" below.

All reinforcing bars deformed except #2 bars and wire mesh. Latest ACI Code and Detailing Manual apply. Unless indicated otherwise in details clear concrete cover to any reinforcing including ties is as follows:

Unformed concrete placed against rough earth.....3"

Formed concrete exposed to earth......2' *All other......1 1/2"

*2" coverage for formed concrete exposed to earth or weather is required for #6 or larger rebar. Smaller clearances permissible for precast or prestressed.

LAP SPLICES IN MASONRY: Shall be 48 diameters.

MESH SPLICES: Wire spacing plus 2 inches.

<u>LAP SPLICES IN CONCRETE</u>: See drawings. Unless noted otherwise, provide the following lap splices:

#3 - 18"lap; #4 - 24"lap; #5 - 30"lap; #6 - 36"lap; #7 - 53"lap

Minimum clear cover for spliced reinforcina is greater than one bar diameter, and minimum clear spacina is greater than two bar

Splice bottom bar over supports and top bar at mid—span only.

Where bars are shown spliced, they may run continuous at Contractor's option.

All splice locations subject to approval. Provide required shop drawings and fabricate after the Architect's review. See Shop Drawing section above. Place rebar per CRSI Manual.

Rebar spacing provided are maximum on center whether stated as "o.c." or not, and all rebar is continuous whether stated as "cont." or not. Provide bent corner rebar to match and lap with horizontal rebar at corners and intersection of walls, beams, bond beams and footings per ACI Manual. Dowel all vertical rebar to foundations. Securely tie all rebar, including dowels, in location before placing concrete or grout.

Where reinforcing is shown continuous thru construction joints, Lenton Form Savers dowel bar splice devices as manufactured by ERICO Products, Inc. (or equivalent) may be used. Sizes and types shall be selected to develop the full tension strength of the bar per ICC Research report.

"Fibermesh" or equivalent independently tested polypropylene fibers may be substituted at a rate of 1.5 pounds per cubic yard of concrete for welded wire fabric in slabs on grade.

MASONRY & MASONRY REINFORCING:

CMU units 1,500 p.s.i. minimum. Block units grade N-1 lightweight or normal weight concrete. Running bond. Mortar type S. Grout 2,000 p.s.i. Mechanical vibrate grout in vertical spaces. Maximum grout lift without cleanouts 4'-0" in block walls. Stay each end and at 8'-0" o.c. vertically each vertical rebar using single wire and loop type ties.

Do not build when air temperature is less than 40 degrees F. Place pipes or conduits in sleeves or hollow unfilled cells only. See Architectural Drawings for expansion or control joints. However, the spacing shall not exceed 24 feet. Do not locate a joint at less than 2'-0'' from bearing of beam or lintel, framing perpendicular to wall.

8" Wall Vertical Reinforcing: In center of grout continuous full height of wall as follows:

1 — #4 U.N.O. at all corners, intersections, wall ends, and each side of joints.

1 - #4 U.N.O. at jambs of opening up to 6'-0" wide.

2 - #4 U.N.O. at jambs of opening up to 12'-0" wide.

1 - #4 U.N.O. at 48" o.c. elsewhere.

Dowel all rebar to foundation.

Horizontal Reinforcing: In minimum 8" deep grouted bond beam two #5, at top of parapets and structurally connected roof and floor levels. Add a mid-height bond beam when the wall is higher than 12'-0" to bearing, or higher than 16'-0" to top of parapet. Provide ladder type #9 joint reinforcing at 16" o.c. Place bond beam bars at roof and floor lines continuous through joints. Cut other bars and joint reinforcing at wall joints. Grout barrier below bond beams shall be continuous wire lath.

Wall Openings: Minimum 1 - #4 in 8" bond beam above and below openings extending 24" beyond jambs. See Lintel Schedule for bond beam requirements for openings larger than 24".

GENERAL: All Wood Frame wall Construction shall comply with the provisions of Chapter 6 of the Current adopted I.C.C. Building Codes.

Do not notch or drill joist, beams or any load bearing member without prior approval of the Project Structural Engineer.

WOOD GRADE: All stress grade lumber construction shall comply with ANSI/NFPA N.D.S. Standards. All lumber, each piece, shall bear the grade stamp of a grading rules agency approved by the American Lumber Standards Committee. Regardless of required grade stamp and certifications, all lumber, each piece, in place in the structure shall be of the original grade specified or better when inspected by a grading agency approved by the A.L.S.C. Grade loss resulting from effects of weathering, handling, storage, re—sawing, or dividing lengths, will be cause for rejection.

BOTTOM PLATES: Shall be Borate treated D.F. #2 or better.

TOP PLATES: Shall be D.F. #2 or better.

BEARING WALL STUDS: Shall be D.F. #2 or better for studs 10'-0" high or less. See Bearing Wall Stud Schedule on Framing Plan.

GLULAM BEAM: Douglas Fir with either 24F—V4 or 24F—V8 combination stress grade per Structural Calcs and Beam Schedule. Materials manufacture and quality control per ANSI/SITC 190.1, except that the moisture content at the time of manufacture shall not exceed 11 percent. Fabrication and handling by AITC licensed fabricator, per latest ANSI/AITC190.1 Standards. Beams to bear an ANSI quality mark.

The fabricator shall have a minimum of five years continuous experience immediately prior to this work. Use wet service condition adhesive. Camber = L/300 where L = span.

Beams to be Architectural, Industrial appearance grade individually or load wrapped. Seal surfaces with sealer coat.

All Exterior Glulam Beams shall be protected from the elements with G.I. flashing and/or wood sealer.

<u>Inspection of Glulam Beams:</u>

1. G.L.B.'s and their plated / bolted connections shall be inspected prior to erection at the job site by an ICC qualified independent inspector with experience in inspecting glulam beams and/or timber framing.

2. Where, as cautioned by American Institute of Timber Construction, tension may occur perpendicular to grain due to wood shrinkage restrained by connection or other reasons, the laminator shall install vertical dowels in glued holes to arrest cracks. Size of dowels and the spacing shall be determined by the laminator for each specific occurrence, regardless of when such cracks occur.

3. All tension lamination finger joints shall be proof—load tested and test results submitted for review.

West Coast Douglas Fir for Posts and 6x beams shall be free of heart centers and select structural. Built-up beams and 4x joists shall be D.F.#1, U.N.O. All other structural framing shall be D.F.#2 or better.

Sprinkler load allowance is 1.5 lbs. per square foot. Suspend sprinklers so that this allowance is not exceeded on any member. Add members if necessary.

See Mechanical and Architectural Drawings for spreaders, metal curbs, or stand to support M.P.E. equipment from purlins or beams or sufficient number of joists.

Double up studs at jambs of openings up to 6'-0''. Provide triple studs at larger opening jambs. Provide horizontal blocking at 48"o.c. vertically staggered, in all bearing walls.

Connections: All framed connections shall be made with framing anchors each side, joist hangers, seats and caps, by Simpson or approved equal, appropriate for the member, for uplift and downwards loads, in accordance with current ICC reports. For nailing see schedule and / or construction detail. Field drill all anchor bolt holes for proper spacing location. Provide cut washers at bolts in wood. Pre-drill all holes for nails larger than 20d. Fasten plywood with 8d common or with .131" x 2" P-nails or No. 13 gage x 1-1/2" long staples, minimum 7/16"o.c. crown. Staples and P-nails shall be installed per ICC-ESR-1539. Fastener spacing shall be 6" at edge supports and 12" at intermediate supports including each of any multiple members, except that the spacing shall not exceed 10" on floor. Minimum edge distance 3/8".

TRUSS JOIST OR WEB JOIST: Fabricator shall be responsible for design using the following loads:

LIVE LOAD: Per plan - 7 day duration - horizontal projection.

DEAD LOAD: Per plan — including weight of joist excluding mechanical units.

Mechanical Equipment: See Mechanical Drawings. Add joist under mechanical equipment.

All construction per current ICC report. Prior to manufacturing, fabricator shall submit design calculations and shop drawings sealed by an Engineer registered in Arizona for review. All permanent and temporary

to require more than 1-1/2" high notch in_supporting wood. WOOD TRUSSES: The manufacturer of the trusses shall be pre-approved for the proposed application, by the building department.

bracing and fastening at bearings shall be by manufacturer. First web member shall be framed so as not

See the plans for the design live load and dead load.

SEE ARCHITECTURAL DRAWINGS FOR PLAN DIMENSIONS, PLATE HEIGHTS, FLOOR ELEVATIONS, AND STAIR RISER / TREAD INFORMATION - PRIOR TO CONSTRUCTION. S.E. CONSULTANTS, INC. IS NOT RESPONSIBLE FOR ARCHITECTURAL RELATED INFORMATION.

GENERAL STRUCTURAL NOTES SCALE: none

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DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION			
	Roof				
Blocking between ceiling joist, rafters or	3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or				
trusses to top plate or other framing below	3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Each end, toe nail			
	2 - 8d common (2 1/2" x 0.131") 2 - 3" x 0.131" nails	Each end, toe nail			
Blocking between rafters or truss not at the	2 - 3" 14 gage staples	Lacif end, toe fiair			
wall top plate, to rafter or truss	2 - 16d common (3 1/2" x 0.162") 3 - 3" x 0.131" nails	End nail			
	3 - 3" 14 gage staples 16d common (3 1/2" x 0.162") @ 6" o.c.				
Flat blocking to truss and web filler	3" x 0.131" nails @ 6" o.c. 3" x 14 gage staples @ 6" o.c.	Face nail			
	3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128"); or				
2. Ceiling joists to top plate	3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	Each joist, toe nail			
Ceiling joists not attached to parallel rafter,	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.128"); or				
laps over partitions (no thrust) (see Section 2308.7.3.1, Table 2308.7.3.1)	4 - 10d box (3 x 0.126), or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Face nail			
4. Ceiling joists attached to parallel rafter (heel joint)	Per Table 2308.7.3.1				
(see Section 2308.7.3.1, Table 2308.7.3.1)	3 - 10d common (3" x 0.148"); or				
5. Collar tie to rafter	4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or	Face nail			
	4 - 3" 14 gage staples, 7/16" crown 3 - 10d common (3" x 0.148"); or				
6. Rafter or roof truss to top plate	3 - 16d box (3 1/2" x 0.135"); or 4 - 10d box (3" x 0.128"); or	Toenail			
(see Section 2308.7.5, Table 2308.7.5)	4 - 3" x 0.131 nails; or 4 - 3" 14 gage staples, 7/16" crown				
	2 - 16d common (3 1/2" x 0.162"); or 3 - 10d box (3" x 0.128"); or				
	3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown; or	End nail			
7. Roof rafters to ridge valley or hip rafters; or roof rafter to 2 - inch ridge beam	3 - 10d common (3 1/2" x 0.148"); or				
	3 - 16d box (3 1/2" x 0.135"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131 nails; or	Toenail			
	4 - 3" 14 gage staples, 7/16" crown				
	Wall	241 6 6 7			
8. Stud to stud (not braced wall panels)	16d common (3 1/2" x 0.162"); 10d box (3" x 0.128"); or	24" o.c. face nail			
((an parior)	3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	16" o.c. face nail			
	16d common (3 1/2" x 0.162"); or	16" o.c. face nail			
Stud to stud and abutting studs at intersecting wall corners (at braced wall panels)	16d box (3 1/2" x 0.135"); or 3" x 0.131 nails; or	12" o.c. face nail			
	3 - 3" 14 gage staples, 7/16" crown	12" o.c. face nail			
10. Built - up header (2" to 2" header)	16d common (3 1/2" x 0.162"); or 16d box (3 1/2" x 0.135")	16" o.c. each edge, face nail			
11. Continuous header to stud	4 - 8d common (2 1/2" x 0.131"); or	Toenail			
The Commission House House House	4 - 10d box (3" x 0.128") 16d common (3 1/2" x 0.162"); or	16" o.c. face nail			
12. Top plate to top plate	10d box (3" x 0.128"); or 3" x 0.131" nails; or	12" o.c. face nail			
	3" 14 gage staples, 7/16" crown	12 O.C. Idee Hall			
13. Top plate to top plate, at end joints	8 - 16d common (3 1/2" x 0.162"); or 12 - 10d box (3" x 0.128"); or	Each side of end joint, face nail (minimum 24" lap splice			
	12 - 3" x 0.131" nails; or 12 - 3" 14 gage staples, 7/16" crown	length each side of each joint)			
14. Bottom plate to joist, rim joist, band joist or block -	16d common (3 1/2" x 0.162"); 16d box (3" x 0.135"); or	16" o.c. face nail			
ing (not at braced wall panels)	3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown	12" o.c. face nail			
45 B. W	2 - 16d common (3 1/2" x 0.162"); or				
15. Bottom plate to joist, rim joist, band joist or block - ing at braced wall panels	3 - 16d box (3 1/2" x 0.135"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	16" o.c. face nail			
	4 - 8d common (2 1/2" x 0.131"); or				
	4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or	Toenail			
16. Stud to top or bottom plate	4 - 3" 14 gage staples, 7/16" crown 2 - 16d common (3 1/2" x 0.162"); or				
	3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or	End nail			
	3 - 3" 14 gage staples, 7/16" crown 2 - 16d common (3 1/2" x 0.162"); or				
17. Top plates, laps at corners and intersections	3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131" nails; or	Face nail			
	3 - 3" 14 gage staples, 7/16" crown 2 - 8d common (2 1/2" x 0.131"); or				
18. 1" brace to each stud and plate	2 - 10d box (3" x 0.128"); or 2 - 3" x 0.131" nails; or	Face nail			
	2 - 3" 14 gage staples, 7/16" crown 2 - 8d common (2 1/2" x 0.131"); or				
19. 1" x 6" sheathing to each bearing	2 - 10d box (3" x 0.128")	Face nail			
20. 1" x 8" and wider sheathing to each bearing	3 - 8d common (2 1/2" x 0.131"); or 3 - 10d box (3" x 0.128")	Face nail			
	Floor				
21. Joist to sill, top plate, or girder	3 - 8d common (2 1/2" x 0.131"); or floor 3 - 10d box (3" x 0.128"); or	Toenail			
21. Joist to Sill, top plate, of glidel	3 - 3" x 0.131" nails; or 3 - 3" 14 gage staples, 7/16" crown	TOGHAII			
22. Rim joist, band joist, or blocking to top plate, sill	8d common (2 1/2" x 0.131"); or 10d box (3" x 0.128"); or	ell o a tarrell			
or other framing below	3" x 0.131" nails; or 3" 14 gage staples, 7/16" crown	6" o.c., toenail			
23. 1" x 6" subfloor or less to each joist	2 - 8d common (2 1/2" x 0.131"); or 2 - 10d box (3" x 0.128")	Face nail			
24. 2" subfloor to joist or girder	2 - 10d box (3 x 0.128) 2 - 16d common (3 1/2" x 0.162")	Face nail			
25. 2" planks (plank & beam - floor & roof)	2 - 16d common (3 1/2" x 0.162")	Each bearing, face nail			
	20d common (4" x 0.192")	32" o.c., face nail at top and bottom stagger on opp. sides			
	10d box (3" x 0.128"); or 3" x 0.131" nails; or	24" o.c., face nail at top and			
26. Build - up girders and beams, 2" lumber layers	3" 14 gage staples, 7/16" crown	bottom stagger on opp. sides			
	And: 2 - 20d common (4" x 0.192"); or 3 - 10d box (3" x 0.128"); or	Ends and at each splice, face nail			
	3 - 10d box (3" x 0.128"); or 3 - 3" x 0.131 nails; or 3 - 3" 14 gage staples, 7/16" crown				
L	1 55 1 ,	1			

27. Ledger strip supporting joists or rafters	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	Each joi	Each joist or rafter, face nail	
28. Joist to band joist or rim joist	3 - 16d common (3 1/2" x 0.162"); or 4 - 10d box (3" x 0.128"); or 4 - 3" x 0.131" nails; or 4 - 3" 14 gage staples, 7/16" crown	End nail	End nail	
29. Bridging or blocking to joist, rafter or truss	2 - 8d common (2 1/2" x 0.131"); or 2 - 10d box (3" x 0.128"); or 2 - 3" x 0.131" nails; or 2 - 3" 14 gage staples, 7/16" crown	Each er	Each end toenail	
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACI	NG AND LOCATION	
Wood structural panels (WSP), subfloor, roof	and interior wall sheathing to framing and particleboard	d wall sheath	ing to framing	
		Edges (inches)	Intermediate supports (inches)	
	6d common or deformed (2" x 0.113") (subfloor and wall)	3	12	
	8d box or deformed (2 1/2" x 0.113") (roof)	3	12	
30. 3/8" - 1/2"	2 3/8" x 0.113" nail (subfloor and wall)	6	12	
	1 3/4" 16 gage staple, 7/16" crown (subfloor and wall)	4	8	
	2 3/8" x 0.113" nail (roof)	3	8	
	1 3/4" 16 gage staple, 7/16" crown (roof)	3	6	
31. 19/32" - 3/4"	8d common (2 1/2" x 0.131"), or 6d deformed (2" x 0.113")	6	12	
31. 13/32 - 3/4	2 3/8" x 0.113" nail; or 2" 16 gage staple, 7/16" crown	4	8	
32. 7/8" - 1 1/4"	10d common (3" x 0.148"), or 8d deformed (2 1/2" x 0.131")	6	12	
	Other exterior wall sheathing			
33. 1/2" fiberboard sheathing ^b	1 1/2" galvanized roofing nail (7/16" head diameter); or 1 1/4" 16 gage staple with 7/16" or 1" crown	3	6	
34. 25/32" fiberboard sheathing ^b	1 3/4" galvanized roofing nail (7/16" head diameter); or 1 1/2" 16 gage staple with 7/16" or 1" crown	3	6	
Wood structural	panels, combination subfloor underlayment to framing	<u> </u>		
35. 3/4" and less	8d common (2 1/2" x 0.131"), or 6d deformed (2" x 0.113")	6	12	
36. 7/8" - 1"	8d common (2 1/2" x 0.131"), or 8d deformed (2 1/2" x 0.131")	6	12	
37. 1 1/8" - 1 1/4"	10d common (3" x 0.148"), or 8d deformed (2 1/2" x 0.131")	6	12	
	Panel siding to framing			
38. 1/2" or less	6d corrosion resistant siding (1 7/8" x 0.106"); or 6d corrosion - resistant casing (2" x 0.099")	6	12	
39. 5/8"	8d corrosion - resistant siding 2 3/8" x 0.128"); or 8d corrosion - resistant casing 2 1/2" x 0.113")	6	6 12	
	Interior paneling			
40. 1/4"	4d casing (1 1/2" x 0.081"), or 4d finish (1 1/2" x 0.072")	6	12	
41. 3/8" 6d casing (2" x 0.099"), or 6d finish (Panel supports at 24 inches)			12	

- a. Common or box nails are permitted to be used except where otherwise stated.
- Nails spaced at 6 inches on center at edges, 12 inches at intermediate supports except 6 inches at supports where spans are 48 inches or more. For nailing of wood structural panel and particleboard diaphragms and shear walls, refer to Section 2305. Nails for wall
- sheathing are permitted to be common, box or casing. c. Common or deformed shank (6d - 2" x 0.113"; 8d - $2\frac{1}{2}$ " x 0.131";
- 10d 3" x 0.148").
- d. Common (6d 2" x 0.113"; 8d $2\frac{1}{2}$ " x 0.131"; 10d 3" x 0.148").
- e. Deformed shank (6d 2" x 0.113"; 8d $2\frac{1}{2}$ " x 0.131"; 10d 3" x 0.148"). f. Corrosion-resistant siding (6d $1\frac{7}{8}$ " x 0.106"; 8d $2\frac{3}{8}$ " x 0.128") or casing (6d 2" x 099"; 8d $2\frac{1}{2}$ " x 0.113") nail.
- g. Fasteners spaced 3 inches on center at exterior edges and 6 inches on center at intermediate supports, when used as structural sheathing. Spacing shall be 6 inches on center on the edges and 12 inches on
- center at intermediate supports for nonstructural applications. h. Corrosion-resistant roofing nails with $\frac{7}{16}$ -inch-diamter head and $\frac{1}{2}$ -inch length for $\frac{1}{2}$ -inch sheathing and $\frac{1}{4}$ -inch length for $\frac{25}{32}$ -inch
- i. Corrosion-resistant staples with nominal $\frac{7}{16}$ -inch crown and $\frac{11}{4}$ -inch length for $\frac{11}{2}$ -inch sheathing and $\frac{11}{2}$ -inch length for $\frac{25}{32}$ -inch sheathing. Panel supports at 16 inches (20 inches if strength axis in the long direction of the panel, unless otherwise marked).
- j. Casing $(1\frac{1}{2}$ " X 0.080") or finish $(1\frac{1}{2}$ " x 0.072") nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- k. Panel supports at 24 inches. Casing or finish nails spaced 6 inches on panel edges, 12 inches at intermediate supports.
- For roof sheathing applications, 8d nails $(2\frac{1}{2}$ " x 0.113") are the
- minimum required for wood structural panels.
- m. Staples shall have a minimum crown width of $\frac{7}{16}$ inch.
- n. For roof sheathing applications, fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports.
- o. Fasteners spaced 4 inches on center at edges, 8 inches at intermediate supports for subfloor and wall sheathing and 3 inches
- intermediate supports for subfloor and wall sheathing and 3 inches on center at edges. 6 inches at intermediate supports for roof sheathing
- center at edges, 6 inches at intermediate supports for roof sheathing.
 p. Fasteners spaced 4 inches on center at edges, 8 inches at
- intermediate supports.

WOOD SHEATHING:

All sheathing shall conform to product standard: A.P.A. P.S. 2—10.

All sheathing shall be APA rated with exterior glue (U.N.O.)

lay up sheathing with face grain perpendicular to supports and stagger joints (U.N.O.) No unblocked panels less than 16" wide shall be used.

Roof Sheathing shall be 1/2" thick plywood, CDX sheathing structural 2 or better (U.N.O.) or 1/2" thick O.S.B. per A.P.A. reg. All roof sheathing shall be gapped 1/8" min., 1/4" max.

I.C.C. approved O.S.B. may be used in lieu of plywood per N.E.R. — #108, Exposure 1. (O.S.B. is <u>NOT</u> allowed at walk—decks)
All wood sheathing shall conform to the following nominal thickness, span index ratio and be attached as follows:

Use:	Min. Thickness:	Span Index:	Edge Attachment:	Intermediate Attachment:
Roof:	1/2"	32 / 16	8d @ 6"o.c.	8d @ 12"o.c.
Wall:	3/8"	24 / 0	8d @ 6"o.c.	8d @ 12"o.c.
Shear Wall:	3/8" (U.N.O.)* * U.N.O. on Shear Wall	24 / 0 Schedule	See Shear Wall Schedule	9

Floor:

w/ It. wt. conc. 1-1/8"

42 / 20

screws @ 6"o.c. screws @ 10"o.c.

w/o It. wt. conc. 1-1/8"

42 / 20

screws @ 6"o.c. screws @ 10"o.c.

48 / 24

Screws at floor sheathing shall be $\#8 \times 2-5/8$ " long for sheathing less than 1-1/2" thick.

Minimum screw diameter = 0.164"

All floor & walkdeck sheathing shall be glued to structural members with an A.P.A. 'AFG-01' qualified gule.

SUPPLEMENTARY NOTES:

Walkdecks:

Provide all temporary bracing, shoring, guying or other means to avoid excessive stresses and to hold structural elements in place during construction.

screws @ 3"o.c.

screws @ 6"o.c.

Any members required to support equipment from the framing shown shall be designed and provided by the equipment Contractor.

For connections, see details. If not shown or noted, minimum connections to be included in cost shall be two 3/4" diameter bolts or 3/16" fillet weld 4" long using 1/4" connection material and detailed to minimize bending in connection. Proceed after clarification through shop drawing submittal.

Expansion bolts in concrete, drawings shall be <u>'Hilti' 'Kwik-Bolt 3'</u> (ESR-1385) or <u>'Hilti' 'TZ'</u> (ESR-1917), or <u>'DeWalt' Wedge-Bolt'</u> (ESR-2526). Embed 3-1/4" minimum for 5/8" diameter bolts. Where spalling is anticipated due to insufficient edge distance, use threaded anchor rod epoxied into drilled hole. Note: All epoxy anchors shall use either 'Simpson' 'SET-XP' (ICC ES ESR-2508 & IAPMO UES ER-265) epoxy in concrete or fully grouted masonry.

'Simpson 'Titen—HD' (ESR—2713) or 'DeWalt' 'Wedge—Bolt' (ESR—1678) 5/8" diameter screw anchors with 3"minimum embedment are acceptable plate anchors.

"Compressible material" shall be roll foam wrap or sponge rubber.

Options and approved substitutions are for Contractor's convenience. He shall be responsible for all changes and additional costs necessary and he shall coordinate all details.

Any engineering design provided by others and submitted for review shall be by an insured Structural Engineer with continuous five years of experience in the type of design submitted.

Unless noted otherwise, details on Structural Drawings are typical as indicated by cuts, references, or titles.

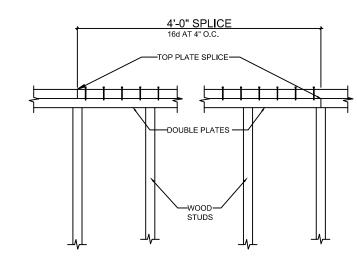
In case of conflicts, more costly requirements govern for bidding. Submit clarification request prior to proceeding with work.

Verify all dimensions with Architectural Drawings.

Contractor shall establish and verify in field all existing conditions affecting new construction. Contact Architect immediately if existing conditions are not as depicted in drawings.

All construction meeting or crossing expansion or shrinkage control joints in framed floors or roofs must have provisions to accommodate the movement or must be delayed until the joint is closed.

Grout other than for filling masonry cells, shall be non-shrink, non-metallic, meeting ASTM C-827, C-191, and C-109, mixed and installed per manufacturer's specifications. Minimum compressive strength 5,000 p.s.i. in two days.



SEE ARCHITECTURAL DRAWINGS FOR PLAN DIMENSIONS, PLATE HEIGHTS, FLOOR ELEVATIONS, AND STAIR RISER / TREAD INFORMATION - PRIOR TO CONSTRUCTION. S.E. CONSULTANTS, INC. IS NOT RESPONSIBLE FOR ARCHITECTURAL RELATED INFORMATION.

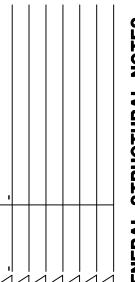
GENERAL STRUCTURAL NOTES

SCALE: none

Consultants, Incal Engineering Consultants

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> ESTABROOK RESIDENC AMERICAN RAN LOT 109 PRESCOTT, ARIZONA



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